



City of Longview

1525 Broadway
Longview, WA 98632
www.ci.longview.wa.us

Agenda

Historic Preservation Commission

Monday, September 30, 2019

5:30 PM

Small Conference Room, City Hall

1. **ROLL CALL**

2. **APPROVAL OF MINUTES**

Minutes of the August 12, 2019 Special Meeting (presented at meeting)

3. **CONSTITUENT COMMENTS**

4. **BOARD MEMBER COMMENTS**

5. **REPORTS**

- Reports from Staff
- Project Longview
- Historic Districts
- Historic Preservation Month

6. **UNFINISHED BUSINESS**

19-000344 CA 2018-3 Harlie's Hoops court at Lake Sacajawea: Final design and finishes for review.

Recommended Action: review structural designs and finishes for appropriateness for the setting and consistency with conceptual approval. Jennifer DesArmo will have a sample of the roofing material to be used.

Motions: Motion to approve project as presented; or motion to approve with conditions; or motion to continue the decision to a future meeting and request more information.

7. NEW BUSINESS

Longview Parks Department Presentations:

- Confirm the placement and mounting pad of the new art piece to be placed at RA Long Park.
- Discuss the replacement of the 2 benches at the Monticello entrance with approved master plan model.
- Explain the CPTED (Crime Prevention Through Environmental Design) reviews that Longview Police completed at RA Long, Lake Sacajawea and Victoria Freeman Parks.
- Present the recommendations that Police Department has for reduction of crime through plant material thinning and removal.

Recommended Action: Review and provide input and guidance to staff; schedule a site visit; agree on items that do or do not need review under a certificate of appropriateness.

Longview Engineering Division:

_Presentation and review of proposed project to paint the Sylvester Apartments in 2020 with a Commerce Department grant. The Sylvester Apartments are attached to the Columbia Theatre and are owned by the City of Longview but operated by the Longview Housing Authority. Historic Inventory information:

<https://mylongview.box.com/s/a2a89azckngmoxsj5ey0ok2hvn7gb7k7>

Recommended Action: provide input to staff on paint project and historic features to preserve, retain and restore.

8. ADJOURNMENT



City of Longview

Agenda Summary

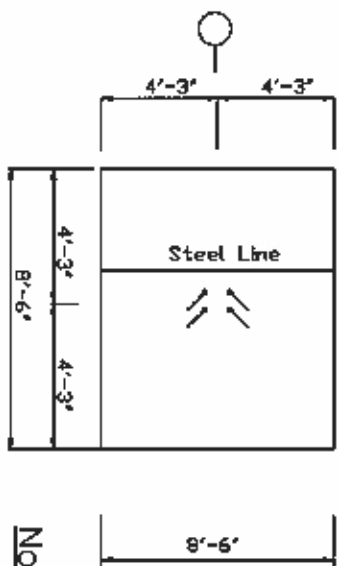
CA 2018-3 Harlie's Hoops court at Lake Sacajawea: Final design and finishes for review.

Recommended Action: review structural designs and finishes for appropriateness for the setting and consistency with conceptual approval. Jennifer DesArmo will have a sample of the roofing material to be used.

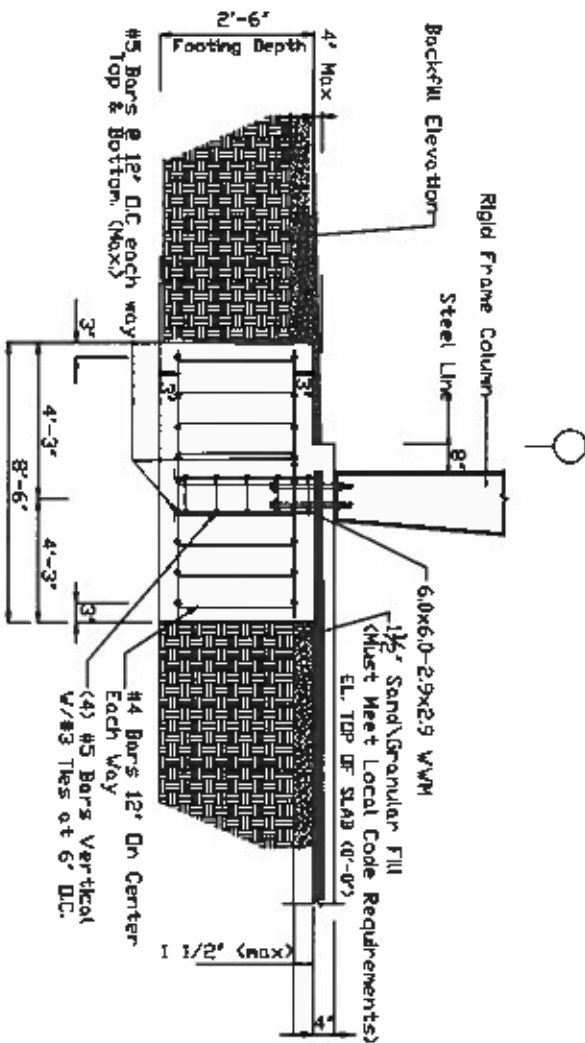
Motions: Motion to approve project as presented; or motion to approve with conditions; or motion to continue the decision to a future meeting and request more information.

Attachments:

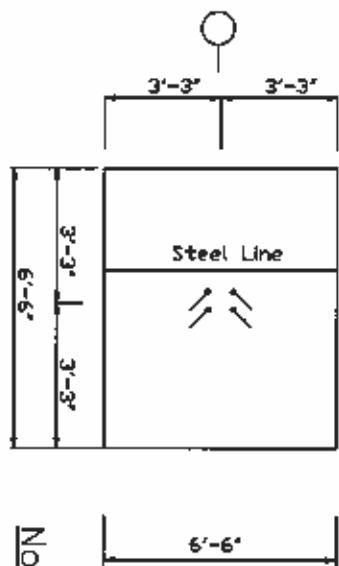
1. New: Harlie's Hoops court structural drawing set
2. Previously approved: concept design rendering
3. Previously approved: SITE PLAN



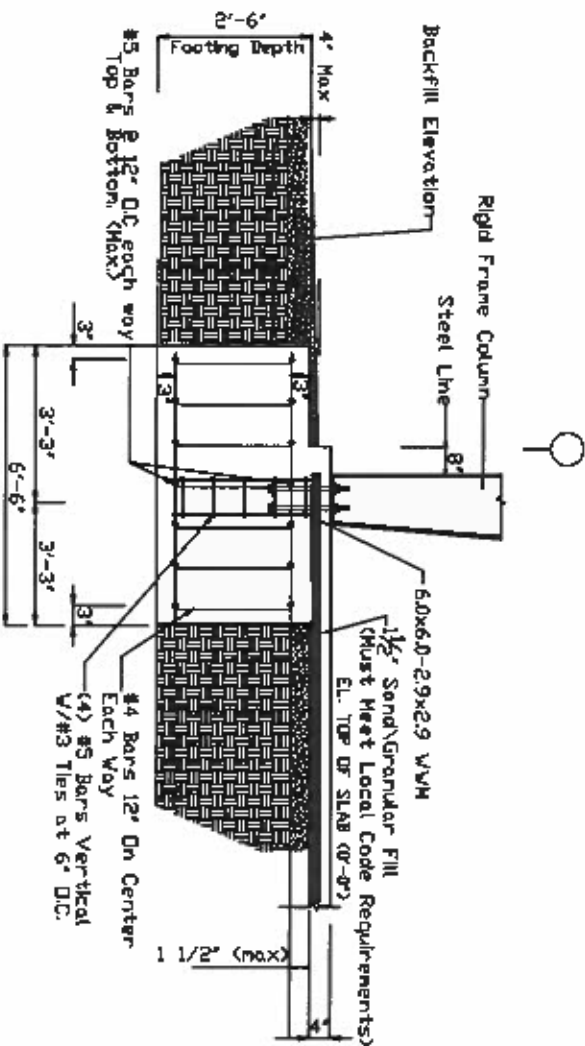
Note: Center Footing Under Column



Section Thru F-01



Note: Center Footing Under Column



Section Thru F-02



EXPIRES 08/20/2020

REVIEWED
By Richard T. Smith at 1:11 pm, Jul 30, 2019



37396 RUBEN LANE
SANDY, OR. 97055
OFFICE: (503)-668-7211 FAX: (503)-668-9026

CUSTOMER: Hardie	
PROJECT: Hardie	
LOCATION: Longview, WA 98632	
DATE: 7/26/19	DRAWN BY: CCJ
JOB NUMBER:	SHEET NUMBER:
R19-021	F02 OF

Note: R.F. and footings not shown for clarity.
G.B. reinforcement shall run through footing.

STRUCTURAL NOTES

STRUCTURAL GENERAL NOTES AND SPECIFICATIONS:

DIVISION 01 - GENERAL

1. The Engineer of Record only assumes responsibility for that which was prepared by the Engineer of Record.
2. Refer to Structural Cover Sheet for applicable structural codes.
3. The structure shown on these drawings is structurally sound only in its completed form. The contractor shall brace all earth, forms, concrete, steel, wood, masonry, to resist gravity, earth, wind and construction loads during construction.
4. Contractor shall exercise proper precaution to verify all existing conditions and layout or work. Immediately notify Engineer of any discrepancies. Contractor is responsible for any error resulting from failure to exercise such precaution.
5. Any discrepancies, errors or omissions discovered in the contract documents shall be brought to the attention of the Engineer before proceeding with related work. Otherwise, the correction of such items is the responsibility of the contractor and/or subcontractor.
6. Where a detail, typical detail, section, typical section or a note is shown for one condition, it shall apply for all like or similar conditions unless otherwise noted.
7. Should structural conflicts occur affecting fit-up of structural material, contractor shall notify engineer. Under no circumstances should structural material be modified to accommodate fit-up without the engineer's approval.

DIVISION 03 - CONCRETE

1. Interior column footings have been designed for placement on original, undisturbed soil or compacted fill material of 1500 PSF minimum bearing capacity. A soils testing laboratory shall verify bearing capacity prior to placing of concrete. In the event, bearing capacity is less than 1500 PSF, notify engineer for further instructions.
2. All fill areas shall be cleared and stripped of organic material under building or paving areas. Proof rolling of existing soil and compaction of fill material to 95% Standard Proctor shall be completed to within 12" of the bottom of the floor slab to a distance of 8'-0" outside of building area before footing excavation is begun. The remaining 12" below the slab shall be compacted to 98% Standard Proctor. Parking areas shall be compacted to a minimum of 90% Standard Proctor. Any engineered structural fill shall be placed in 8' lifts, maximum.
3. Structural concrete shall be as follows, unless otherwise noted, 28 day minimum compressive strength:
 - a) Footings & Foundations: 3000 PSI
 - b) Floor Slab: (as noted on drawings)Slump attained shall be 4" (+/- 1"). Concrete for masonry filled cells may be placed with 8" to 11" slump. Unless noted otherwise, details of concrete reinforcement and accessories shall be in accordance with ACI 315, Manual of Standard Practice for Detailing Reinforced Concrete Structures and CRSI MSP-1, Manual of Standard Practice, latest edition.
5. Reinforcing steel shall conform to ASTM A615, grade 60, and ASTM A616.
6. Unless otherwise noted, reinforcing lap splices shall be ACI Class B splices using the following lap lengths:

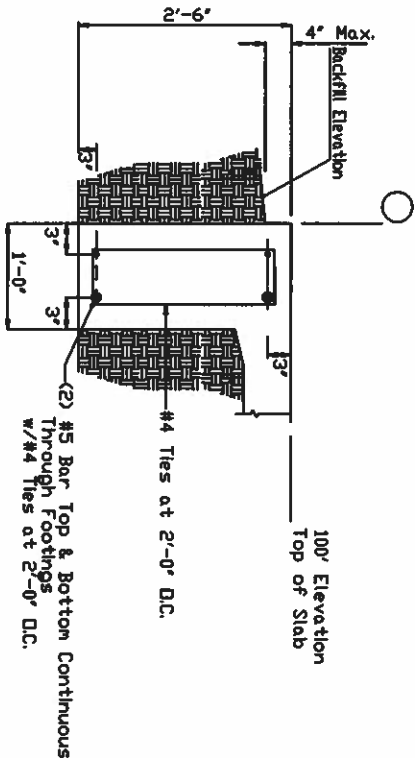
#4	24"
#5	30"
#6	36"
#7	42"
7. All welded reinforcing steel shall be ASTM A706 and be free of oil, scale, and rust. Welding of bars shall conform to ANSI/AWS D1.4 Structural Welding Code - Reinforcing Steel.
8. Wire mesh shall conform to ASTM A185, minimum lap to be 6 inches.
9. Provide corner bars at corners of concrete walls and footings. Size and spacing of bars shall match size and spacing of longitudinal bars in walls or footings.
10. Concrete slab and design criteria shall be as noted on the structural plans.
11. Place 6 mil (nom) polyethylene vapor barrier under all building slabs on grade, lap 12" minimum.
12. Concrete test reports shall be maintained at job site at all times and available for review by Building Inspectors.
13. Slabs on grade shall be placed using strip placement. Sawn joints (noted as S.J. foundation plan) shall be cut as soon as possible after slab is able to support weight of saw and be cut without raveling. Sowing shall be performed within 4 to 12 hours and absolutely before 24 hours has passed from time after first placement. Saw joint nearest midpoint of strip first and then half-way between cuts next.

Concrete (Cont'd)

14. Unless noted otherwise, minimum clear cover for reinforcement shall be as follows:
 - a. concrete cast against earth-3"
 - b. formed concrete exposed to earth or weather-1 1/2" for #5 bars and smaller, 2" for #6 bars and larger.
15. Immediately upon final troweling of slabs, coat with curing compound which meets or exceeds ASTM C-309 "Liquid Membrane-Forming Compounds for Curing Concrete." Coverage shall not be less than 1 gallon per 160 square feet of slab area or more if recommended by curing compound manufacturer (minimum of 8 to 10 mils thick). Floors shall be finished to FF 35 and FL 25, minimum. Do not add calcium chloride or other salty compounds to concrete without specific authorization by Structural Engineer. In no case shall calcium chloride exceed 1 Percent.
18. Use Portland Cement Type I or II conforming to ASTM C150-92. Aggregates shall be normal weight conforming to ASTM C33.
19. For every vertical or horizontal bar discontinued by an opening, one bar (min. of two bars) shall be added at the side of the openings. Slabs at corners of openings, cut-outs and penetrations shall be reinforced with 2-#4 (3-0" long) diagonals unless otherwise noted. Pipes, ducts, conduits, etc. shall not be placed in slabs unless approved by the engineer. (Place all pipes below slab).
21. Concrete exposed to weather shall be air-entrained 30% to 50%. Interior slabs shall have air content of 0% to 3% maximum.
22. All concrete anchorage shall be attained by using cast in place place headed studs with respective washers to increase the concrete breakout strength and shall conform to ASTM F1554-99 Standard Specification for Anchor Bolts & ASTM A307-00 Standard Specification for Carbon Steel Bolts and Studs, 60,000 psi Tensile Strength.
23. Minimum edge distances for cast-in-place headed anchors that will be torqued, the minimum edge distances shall be 6 times the diameter. Minimum edge distances for cast-in-place headed anchors that will not be torqued, the minimum edge distances shall be 2" when cast in concrete exposed to earth or weather.
24. For cast-in-place threaded anchors, a metal or plywood template mounted above the surface of the concrete with nuts on each side of the template should be used to hold the anchors in a fixed position while the concrete is placed, consolidated, and hardened. Post-installed anchors shall be installed in accordance with the manufacturer's installation instructions.

DESIGN CRITERIA

Width (ft)	=	60.0
Length (ft)	=	90.0
ave Height (ft)	=	19.0/ 19.0
Roof Slope (rise/run)	=	3.0/ 3.0
Dead Load (psf)	=	25
Collateral Load (psf)	=	3.0
Roof Live Load (psf)	=	20.0
Snow Load (psf)	=	25.0
Wind Speed (mph)	=	130.0
Wind Code	=	IBC 15
Exposure	=	C
Closed/Open	=	P
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Design Category	=	D
Seismic Coeff (F _{max})	=	1.08



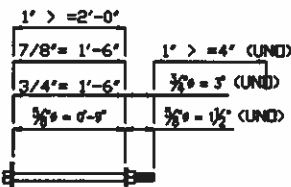
SECTION A



Keyed Construction Joint



SECTION F



Typ. Anchor Bolts

Headed Studs w/1/2 washers conforming to ASTM Grade 36 Standard Specification for Anchor Bolts



EXPIRES 08/20/2020

REVIEWED

By Richard T Smith at 1:11 pm, Jul 30, 2019



37396 RUBEN LANE
SANDY, OR. 97055
OFFICE: (503)-668-7211 FAX: (503)-668-9026

CUSTOMER: Harlies	
PROJECT: Harlie	
LOCATION: Longview, Wa 98632	
DATE: 7/26/19	DRAWN BY: CCJ
JOB NUMBER:	SHEET NUMBER:
R19-021	F03 OF F03

DRAWING PACKAGE

BASIC BUILDING SIZE: 60'-0" x 90'-0" x 19'-0" ROOF PITCH: 3.0:12
ROOF COVER: PBR COLOR: NEED COLOR GA: 26
WALL COVER: PBR COLOR: NEED COLOR GA: 26
TRIM COLORS: GABLE: NEED COLOR WALL: NEED COLOR
TRIM COLORS: CORNER: NEED COLOR GUTTER:
SNOW LOAD: 25 PSF. CLOSURE: Partial
LIVE LOAD: 20/00 COLL. LOAD: 3 ADDITIONAL LOAD: N/A
SEISMIC DESIGN CATEGORY: D IMPORTANCE: 100
WIND LOAD: = 130 mph) Exposure: C DESIGN CODE/ED: IBC 15

CUSTOMER: Harlie's Angels
ADDRESS: P.O. Box 1834
Castle Rock, Wa 98611
PHONE: FAX:
PROJECT: Harlie's Hoops BB Cover
LOCATION: Longview, Wa 98632

GENERAL NOTES:

1. MATERIALS. ASTM DESIGNATION.
- | | | |
|---------------------------|-------------|------------------------|
| HOT ROLLED BAR | A529 | Fy =50 ksi MIN. |
| STRUCTURAL STEEL SHEET | A570 | Fy =50 ksi MIN. |
| STRUCTURAL STEEL PLATE | A572 | Fy =50 ksi MIN. |
| COLD FORMED SHAPES | A570 | Fy =55 ksi MIN. |
| COLD FORMED SHAPES (GALV) | A653 | Fy =55 ksi MIN. |
| ROOF & WALL SHEETING | A792 | Fy =80 ksi MIN. 26 Ga. |
| ROOF & WALL SHEETING | A792 | Fy =40 ksi MIN. 24 Ga. |
| BOLTS | A307 & A325 | |
- WEB STEEL BUILDINGS NW RESERVES THE RIGHT TO SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.

2. A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325 UNLESS NOTED OTHERWISE. HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE 'TURN OF THE NUT' METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS.' A325 BOLTS SHALL BE INSTALLED WITHOUT WASHERS WHEN TIGHTENED BY THE 'TURN OF THE NUT' METHOD. ALL BOLTED CONNECTIONS FOR SHEAR / BEARING CONNECTION TYPE WITH BOLT THREADS EXCLUDED FROM THE SHEAR PLANE SHALL BE SNUG TIGHT ONLY.

3. ALL STRUCTURAL STEEL TO RECEIVE A RUST INHIBITIVE PRIMER. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.

4. PRE-FORMED CLOSURE STRIPS ARE PROVIDED AT THE EAVE BENEATH THE ROOF SHEETS, AT THE ENDWALLS BETWEEN THE RAKE TRIM AND THE END-WALL SHEETS AND AT THE SIDEWALLS BETWEEN THE EAVE TRIM AND THE SIDEWALL SHEETS.

5. ALL BRACING SHOWN AND PROVIDED BY WEB STEEL BUILDINGS NW FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING, AND TO PROCURE AND INSTALL AS NEEDED.

CUSTOMER RESPONSIBILITIES

1. IT IS THE RESPONSIBILITY OF THE ERECTOR TO PERFORM THE WORK IN A SAFE MANNER AND TO OBEY ALL APPLICABLE SAFETY CODES.
2. IT IS THE RESPONSIBILITY OF THE CUSTOMER TO ENSURE THAT ALL PROJECT PLANS, SPECIFICATIONS AND APPLIED DESIGN LOADS, FULLY MEET THE PROPOSED INTENT AS WELL AS COMPLY WITH ALL APPLICABLE REQUIREMENTS OF ANY GOVERNING AUTHORITIES, AND OBTAIN THE APPROPRIATE APPROVALS AND/OR PERMITS AS MAY BE REQUIRED FROM CITY, COUNTY, STATE OR FEDERAL AGENCIES.
3. THE BUILDING WILL BE SUPPLIED EXACTLY AS THIS DRAWING PACKAGE INDICATES. IT IS THE RESPONSIBILITY OF THE CUSTOMER TO ENSURE THAT THE BUILDING BEING SUPPLIED MEETS THE PROJECT REQUIREMENTS.
4. IT IS THE RESPONSIBILITY OF THE CUSTOMER FOR OVERALL PROJECT COORDINATION, INTERFACE, COMPATIBILITY AND DESIGN CONCERNING ANY MATERIALS NOT SUPPLIED BY WEB STEEL BUILDINGS NW WITH THE BUILDING TO BE SUPPLIED BY WEB STEEL BUILDINGS NW. THE SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BY THE CUSTOMER PRIOR TO RELEASE FOR FABRICATION, OR THE DESIGN ASSUMPTIONS OF WEB STEEL BUILDINGS NW WILL GOVERN.

5. IT IS THE RESPONSIBILITY OF THE CUSTOMER TO VERIFY ALL DESIGN CRITERIA, INCLUDING DEFLECTIONS, AND NOTIFY WEB STEEL BUILDINGS NW OF ANY DISCREPANCIES OR INCONSISTENCIES PRIOR TO RELEASE FOR FABRICATION.
6. THE CUSTOMER IS RESPONSIBLE FOR THE ACCURATE SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH 'FOR CONSTRUCTION' DRAWINGS ONLY. (SEE ANCHOR BOLT PLAN FOR ADDITIONAL FOUNDATION AND ANCHOR BOLT NOTES.)

7. UNLESS SPECIFICALLY STATED OTHERWISE WITHIN THE WEB STEEL BUILDINGS NW PURCHASE AGREEMENT, THE FOLLOWING SHALL GOVERN:

- a. THIS BUILDING DRAWING PACKAGE AND ANY ACCOMPANYING CALCULATIONS REPRESENT THE PROJECT AS IT WILL BE SUPPLIED. THE BUILDING WILL BE SUPPLIED EXACTLY AS THE DRAWINGS INDICATE.
- b. ANY FIELD MODIFICATION OR ALTERATION, OR THE ATTACHMENT OF ANY EQUIPMENT OR COMPONENT TO THE STRUCTURE DESIGNED HEREIN, OR THE OMITTED INSTALLATION OF ANY PARTS OR PIECES, WITHOUT THE EXPRESS WRITTEN APPROVAL OF WEB STEEL BUILDINGS NW SHALL VOID ANY AND ALL WARRANTIES.

CUSTOMER APPROVAL:

ALL INFORMATION CONTAINED HEREIN HAS BEEN REVIEWED AND FOUND TO BE CORRECT AND CONSISTENT WITH MY INTENT AND PURPOSE. I REQUEST THAT WEB STEEL OF WASHINGTON PROCEED WITH FABRICATION. I UNDERSTAND AND ACCEPT ALL CUSTOMER RESPONSIBILITIES.

SIGNED: DATE:

DRAWING RECORD		☒		
DESCRIPTION	PAGE NO.	☒		
FOUNDATION PLAN	SHEET -	☒		
ANCHOR BOLT PLAN	SHEET - A1-A3	☒		
CROSS SECTION	SHEET - E1-E2	☒		
ROOF FRAMING PLAN	SHEET - E3	☒		
SIDEWALL ELEVATIONS	SHEET - E4-E5	☒		
ENDWALL ELEVATIONS	SHEET - E6-E7	☒		
ROOF FRAMING DETS.	SHEET - D1	REV	DATE	REVISION
BRIDGING DETAILS	SHEET - D2			
E.W. FRAMING DETS.	SHEET - D3			
PORTAL DETAILS	SHEET - E4-E5			

WEB STEEL

of Washington, LLC

37396 RUBEN LANE
SANDY, OR. 97055
OFFICE: (503)-668-7211 FAX: (503)-668-9026 R19-021

REVIEWED

By Richard T. Smith at 10:02 am, Jul 24, 2019

EXP. 08/20/2020

RICHARD T. SMITH

REGISTERED PROFESSIONAL ENGINEER

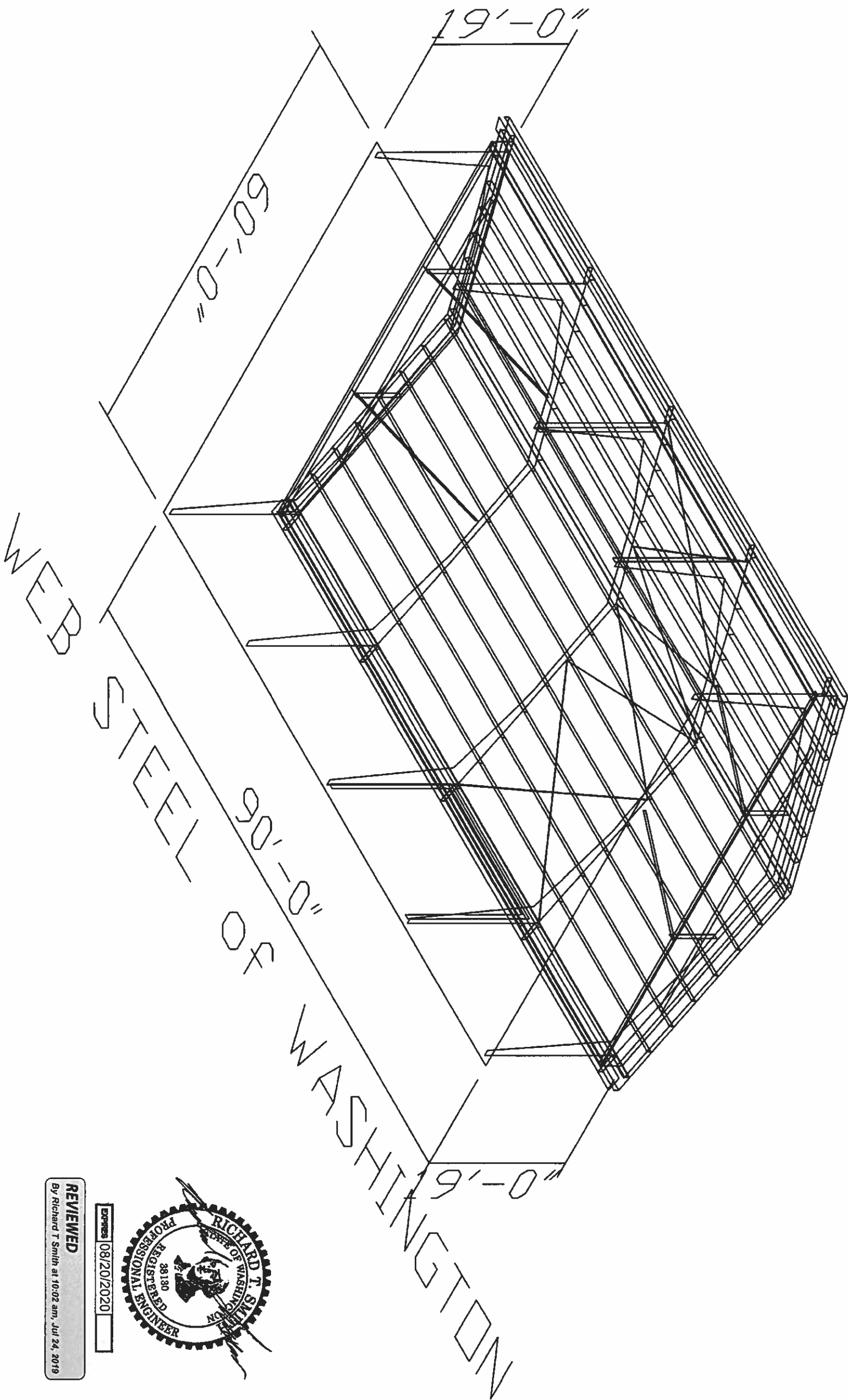
36130

STATE OF WASHINGTON

IMPORTANT NOTE

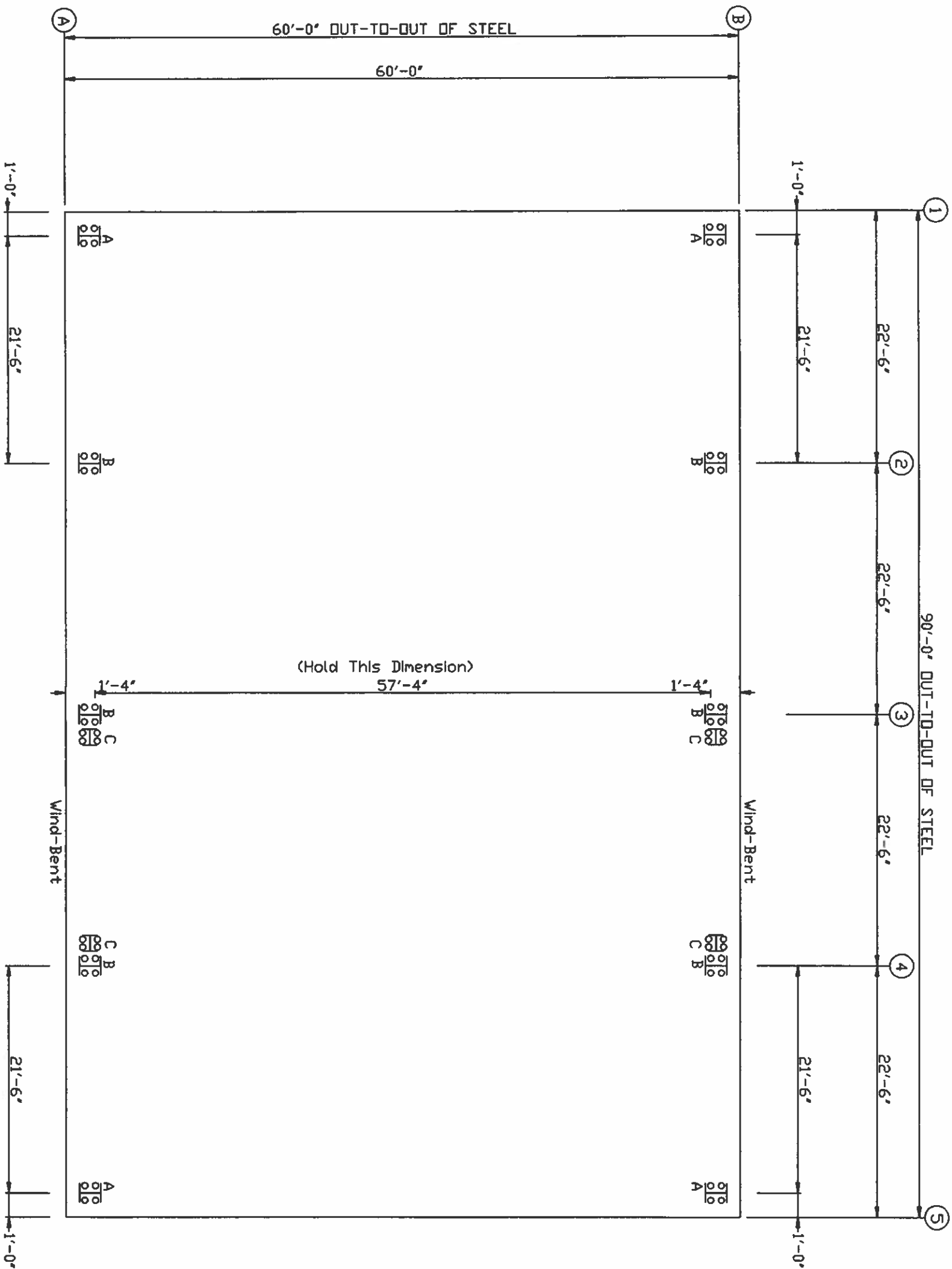
IN NO CIRCUMSTANCES SHALL THE ENGINEERS OF WEB STEEL OF WASHINGTON OR ITS CONTRACT ENGINEER(S) BE DEEMED AS THE ENGINEER OF RECORD FOR THIS PROJECT UNLESS OTHERWISE NOTED.

FOUNDATIONS ARE INCLUDED: ☒ EXCLUDED: ☐



REVIEWED
By Richard T. Smith at 10:02 am, Jul 24, 2019

Expires 08/20/2020

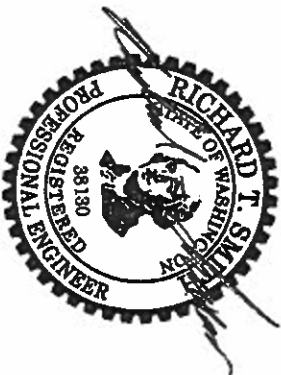


ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)



37396 RUBEN LANE
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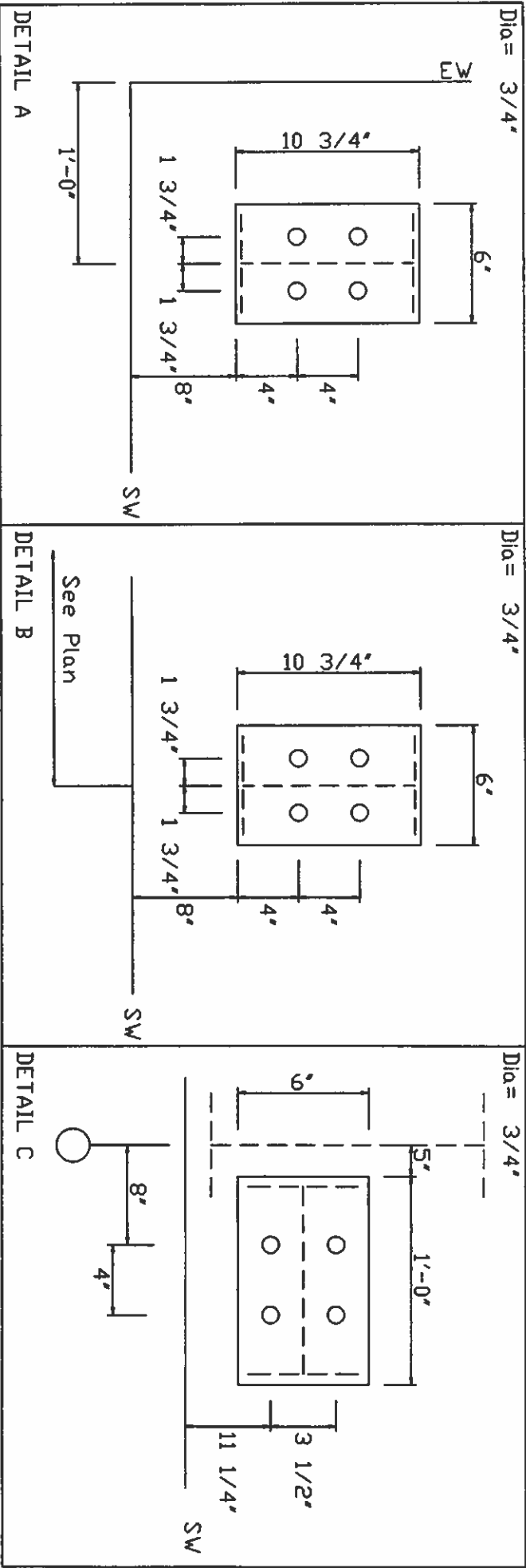


Expires 08/20/2020

REVIEWED

By Richard T Smith at 10:02 am, Jul 24, 2019

CUSTOMER:	Harlies
PROJECT:	Harlie
LOCATION:	Lonview, Wg 98632
DATE:	7/22/19
DRAWN BY:	JSS
JOB NUMBER:	SHEET NUMBER: A1
R19-021	



Expires 08/20/2020

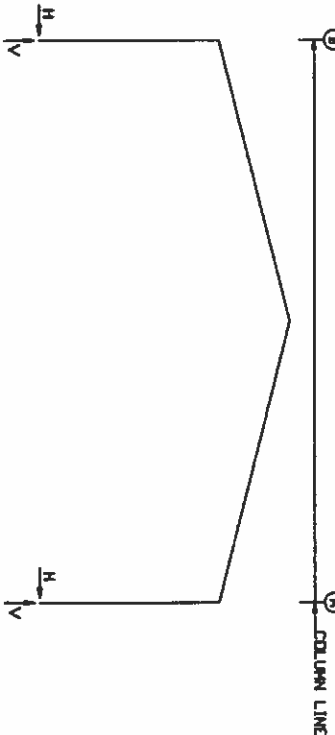
REVIEWED
By Richard T Smith at 10:02 am, Jul 24, 2019



WEB STEEL, LLC
Of Washington, LLC
37396 RUBEN LANE
SANDY, OR 97055
OFFICE: (503)-668-7211 FAX: (503)-668-9026

CUSTOMER: Hardie's Angels	
PROJECT: Hardie's Hoops Basketball Cover	
LOCATION: Longview, WA 98632	
DATE: 7/22/19	DRAWN BY: JSS
JOB NUMBER:	SHEET NUMBER: A2
R19-021	

FRAME LINES: 1 2 3 4 5



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)				V		Bottom		Base Plate(s)		Graft	
		Load	Max	V	Id	H	V	Qty	Dia	Width	Length		Thick
1m	B	1	6.9	15.9	2	-5.7	-12.0	4	0.750	6.000	10.75	0.500	0.0
1m	A	3	-6.9	-12.0	1	-6.9	15.9	4	0.750	6.000	10.75	0.500	0.0
1m	Frame lines: 1 5												

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)				V		Bottom		Base Plate(s)		Graft (m)	
		Load Id	Max H	Vmax	Id H	Vmin	Qty	Dia	Width	Length	Thick		
2m	B	1	10.7	24.2	2	-7.3	-16.5	4	0.750	6.000	10.75	0.500	0.0
2m	A	3	7.3	-16.5	1	-10.7	24.2	4	0.750	6.000	10.75	0.500	0.0
2m	Frame lines: 2 4												

ANCHOR BOLT SUMMARY

Qty	Locate	Dia	Type	Proj
0.40	Frame	3/4"	A307	3.00
0.16	WindCol	3/4"	A307	3.00

WIND BENT REACTIONS

Wind		Seismic		Bottom		Base Plate(s)		Graft					
Load	Max	Id	H	Qty	Dia	Width	Length	Thick	0.0				
1m	B	1	10.7	24.2	2	-7.3	-16.5	4	0.750	6.000	10.75	0.500	0.0
1m	A	3	7.3	-16.5	1	-10.7	24.2	4	0.750	6.000	10.75	0.500	0.0
1m	A	1	-10.7	24.2	3	7.3	-16.5						
1m	Frame lines: 1 5												

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead		Live		Snow		Wind Left		Wind Right			
		Load	Max	Id	H	Qty	Dia	Width	Length	Thick	Thick		
1m	B	1	12	3.0	1.1	6.1	6.8	14.4	8.4	19.1	13.4	2.9	-3.4
1m	A	3	-12	3.0	-1.1	-6.1	-6.8	-14.4	-8.4	-19.1	-13.4	-2.9	3.4
1m	A	1	-6.9	15.9	3	5.7	-12.0						
1m	Frame lines: 1 5												

GENERAL NOTES

- Foundation and building construction are not the responsibility of WEB STEEL BUILDINGS.
- The building reaction data reports the loads which this building places on the foundation.
- Anchor bolts shall be accurately set to a tolerance of +/- 1/8" in both elevation and location. Anchor bolts are sized based on the steel strength of ASTM F1554 Grade 36 material, and require flat washers. Anchorage of the anchor bolts is the responsibility of the foundation designer.
- Column base plates are designed not to exceed bearing pressure of 875 pounds per square inch.
- Anchor bolts, and embedded items are not provided by WEB STEEL BUILDINGS.
- Columns, and should be considered suitable when loads are applied from the opposite side.

SEISMIC ENGINEERING DESIGN CRITERIA

Seismic Design Category:	D	
Soil Site Class:	D	
Mapped Spectral Response:	$S_s = 0.9680$	$S_1 = 0.4490$
Design Spectral Response:	$S_{ds} = 0.7181$	$S_{d1} = 0.4643$
Ground Snow Load	$P_g = 25.0000$	psf
Sloped Roof Snow Load	$P_s = 25$	psf
Snow Exposure Factor:	$C_e = 1.0000$	
Thermal Factor:	$C_t = 1.00$	

SNOW LOADING DESIGN CRITERIA

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 60.0
 - Length (ft) = 150.0
 - Roof Slope (rise/run) = 3.0/12.0
 - Dead Load (psf) = 2.5
 - Collateral Load (psf) = 3.0
 - Live Load (psf) = 20.0
 - Snow Load (psf) = 25.0
 - Wind Speed (mph) = 150
 - Wind Code = IBC 15
 - Exposure Category = B
 - Importance Wind = 100
 - Importance Seismic = 100
 - Seismic Zone = D
 - Seismic Coeff (Rasx) = 1.08
- Loading conditions are:
 - 1 Dead+Collateral+Snow+Side+Snow
 - 2 Dead+Collateral+Snow
 - 3 Dead+Collateral+Wind Left
 - 4 Dead+Collateral+Wind Right

BUILDING BRACING REACTIONS

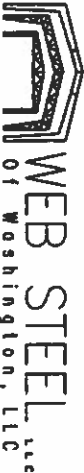
Frm Line	Col Line	Reaction (k)		Panel Shear (lb/ft)	Note
		Load	Max		
1m	B	1	3.4		(a)
1m	A	3	3.4		(a)
1m	A	1	3.4		(a)
1m	Frame lines: 1 5				



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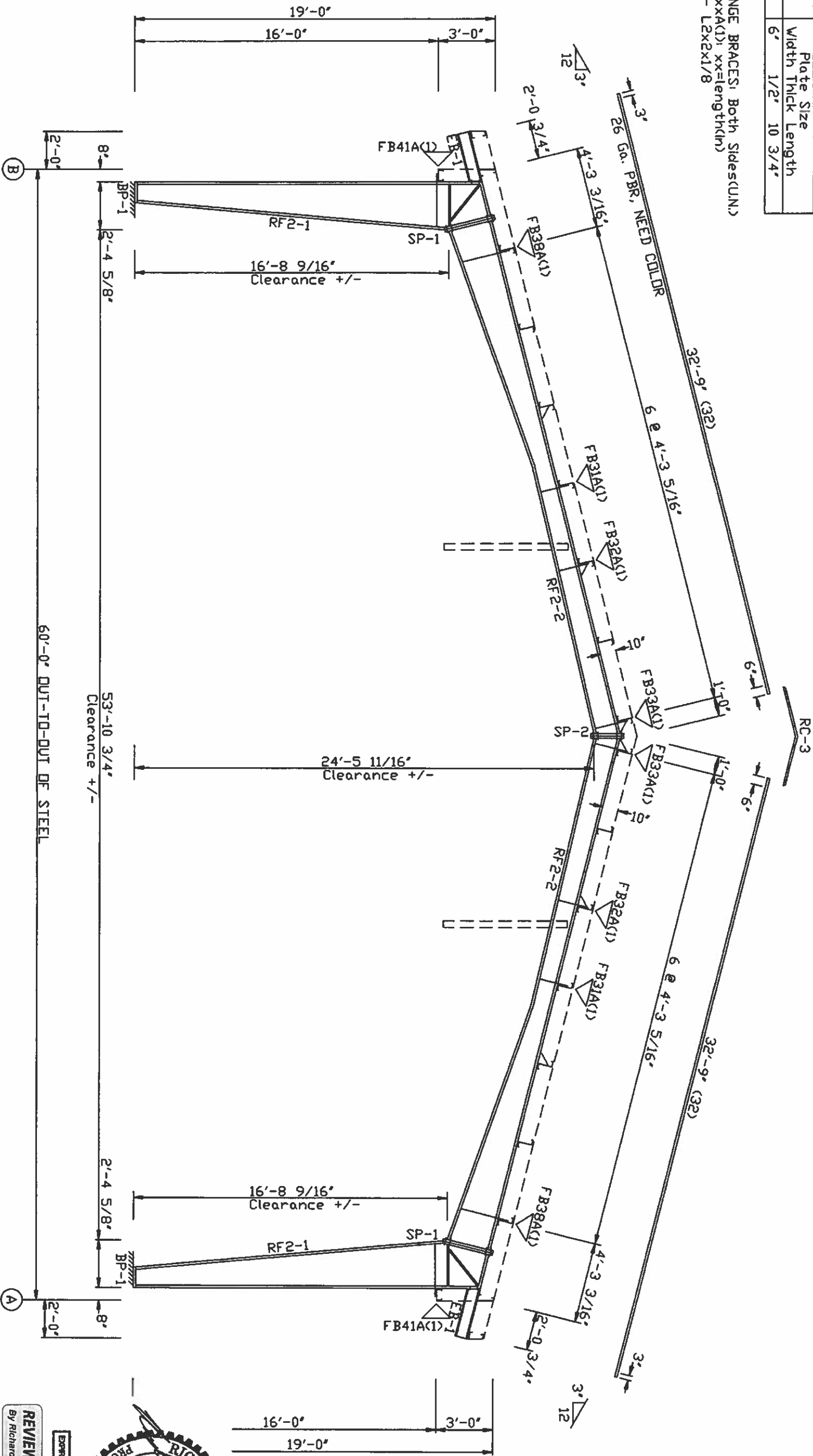
CUSTOMER: Harlie's Angels
PROJECT: Harlie's Hoops Basketball Cover
LOCATION: Longview, Wa 98632
DATE: 7/22/19
DRAWN BY: JSS
JOB NUMBER: R19-021
SHEET NUMBER: A3

SPICE PLATE & BOLT TABLE										
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0		A325	3/4"	2 1/4"	6"	5/8"	2'-9 1/4"
SP-2	4	4	0		A325	3/4"	1 3/4"	6"	3/8"	1'-9 3/4"

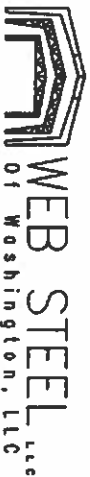
BASE PLATE TABLE		
Col	Plate Size	Width Thick Length
BP-1	6"	1/2" 10 3/4"

FLANGE BRACES, Both Sides(CUN.)
 FBxxx(A1), xx=length(in)
 A - L2x2x1/8

MEMBER TABLE					
Mark	Weight	Web Depth	Web Plate	Outside Flange	Inside Flange
RF2-1	463	10.0/28.0	0.135	1/4" x 6 x 218.9	3/8" x 6 x 196.7
RF2-2	599	25.0/10.0	0.188	1/4" x 6 x 240.0	1/4" x 6 x 160.1
		10.0/13.0	0.135	1/4" x 6 x 99.4	1/4" x 6 x 176.7



FRAME CROSS SECTION: FRAME LINE 1 5



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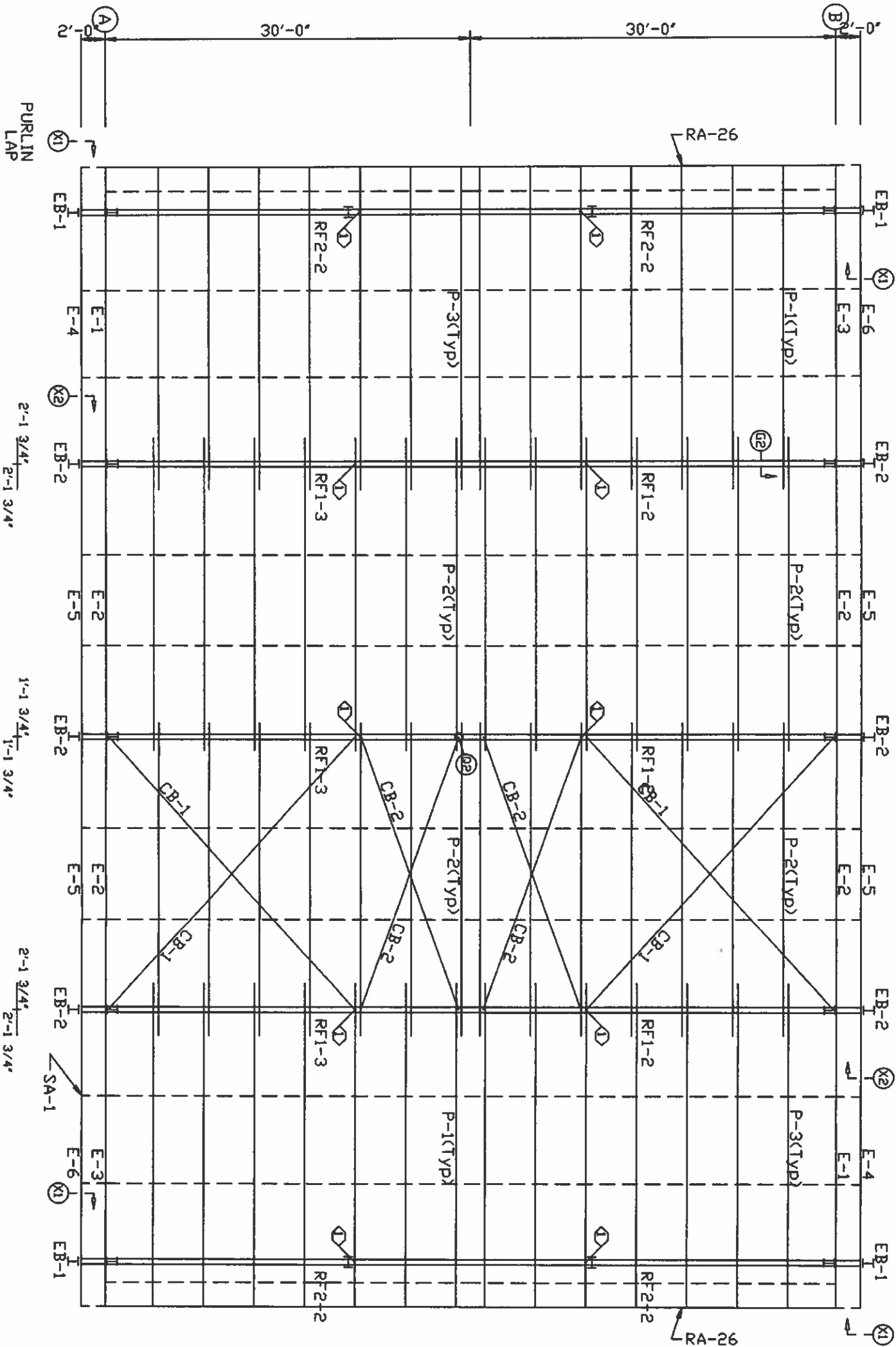
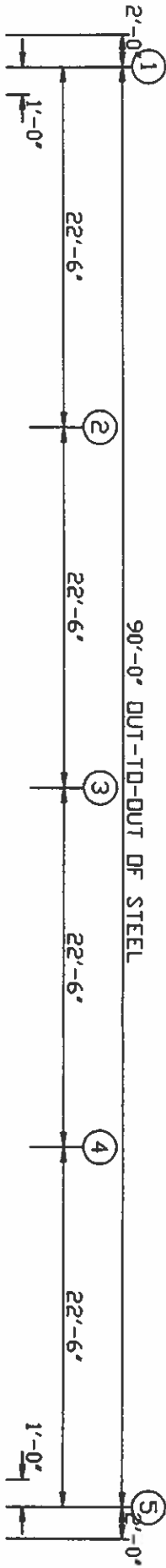
EXPRESS 08/20/2020

CUSTOMER: Harlies Angels
 PROJECT: Harlies Hoops Basketball Cover
 LOCATION: Longview, Wa 98632
 DATE: 7/22/19
 DRAWN BY: JSS
 JOB NUMBER:
 SHEET NUMBER: E2

EXTENSION/CANOPY BOLTS			
ROOF PLAN			
MARK	QUAN	TYPE	DIA. LENGTH
EB-1	4	A325	1/2" 1 1/4"
EB-2	4	A325	1/2" 1 3/4"

SPECIAL BOLTS			
ROOF PLAN			
O.I.D	QUAN	TYPE	DIA. LENGTH WASH
1	4	A307	1/2" 1 1/4" 0

MEMBER TABLE			
ROOF PLAN			
MARK	PART	W	X
EB-1	W8X10		
EB-2	W8X10		
P-1	10214E25		
P-2	10214E25		
P-3	10214E25		
E-1	10E3DL14		
E-2	10E3DL14		
E-3	10E3DL14		
E-4	10E3DL14		
E-5	10E3DL14		
E-6	10E3DL14		
CB-1	FWC375		
CB-2	FWC250		



RC-3
(32)

32'-9" (32)



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ROOF SHEETING
PANELS: 26 Ga. PBR-PANEL
NEED COLOR

ROOF FRAMING PLAN



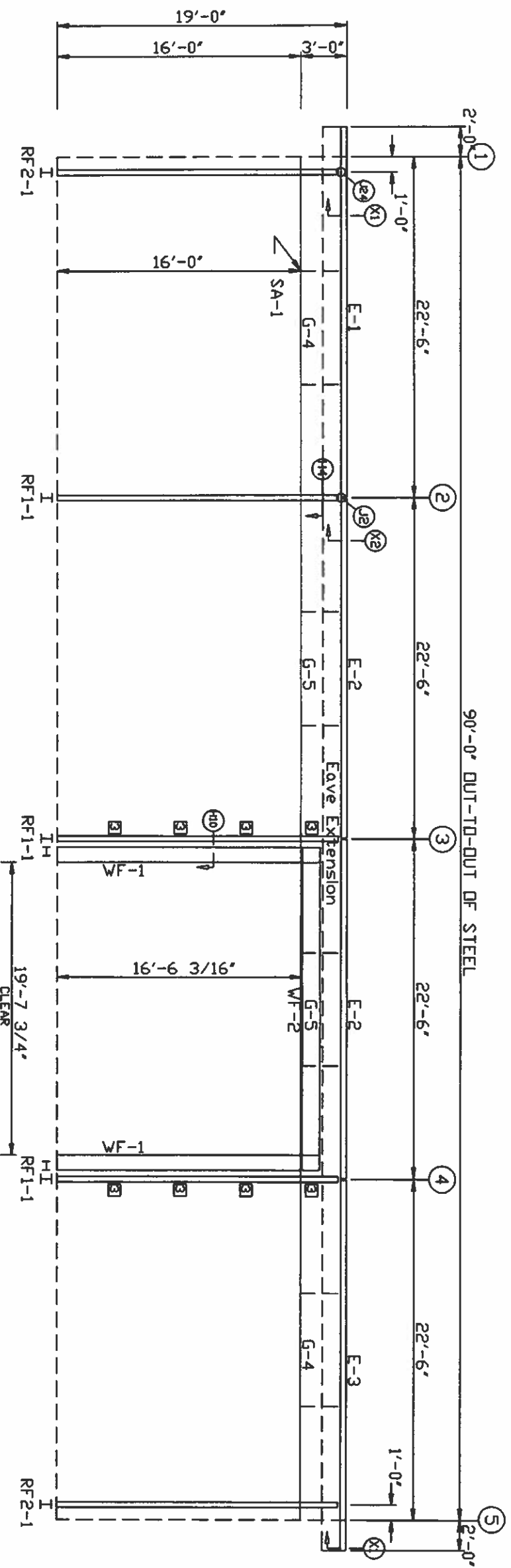
37396 RUBEN LANE
SANDY, OR. 97055
OFFICE: (503)-668-7211 FAX: (503)-668-9026

CUSTOMER: Harlie's Angels
PROJECT: Harlie's Hoops Basketball Cover
LOCATION: Longview, WA 98652
DATE: 7/22/19
DRAWN BY: JSS
JOB NUMBER: R19-021
SHEET NUMBER: E3

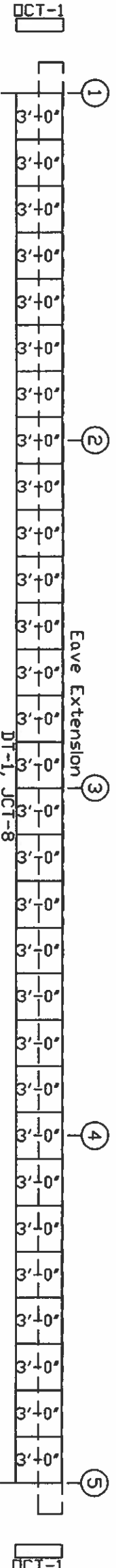
BOLT TABLE		
FRAME LINE A	QUAN	TYPE DIA LENGTH
WF-1 - WF-2	8	A325 7/8" 2 3/4"
WF-1 - RF1-1	16	A325 5/8" 1 3/4"

MEMBER TABLE		
FRAME LINE A	PART	
WF-1	W12X16	
WF-2	W14X30	
E-1	10E3DL14	
E-2	10E3DL14	
E-3	10E3DL14	
G-4	8C16E25	
G-5	8C16E25	

CONNECTION PLATES		
FRAME LINE A	CID	MARK/PART
	3	PC-1



FRONT SIDEWALL FRAMING: FRAME LINE A



FRONT SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. PR - NEED COLOR



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EXP00023 08/20/2020



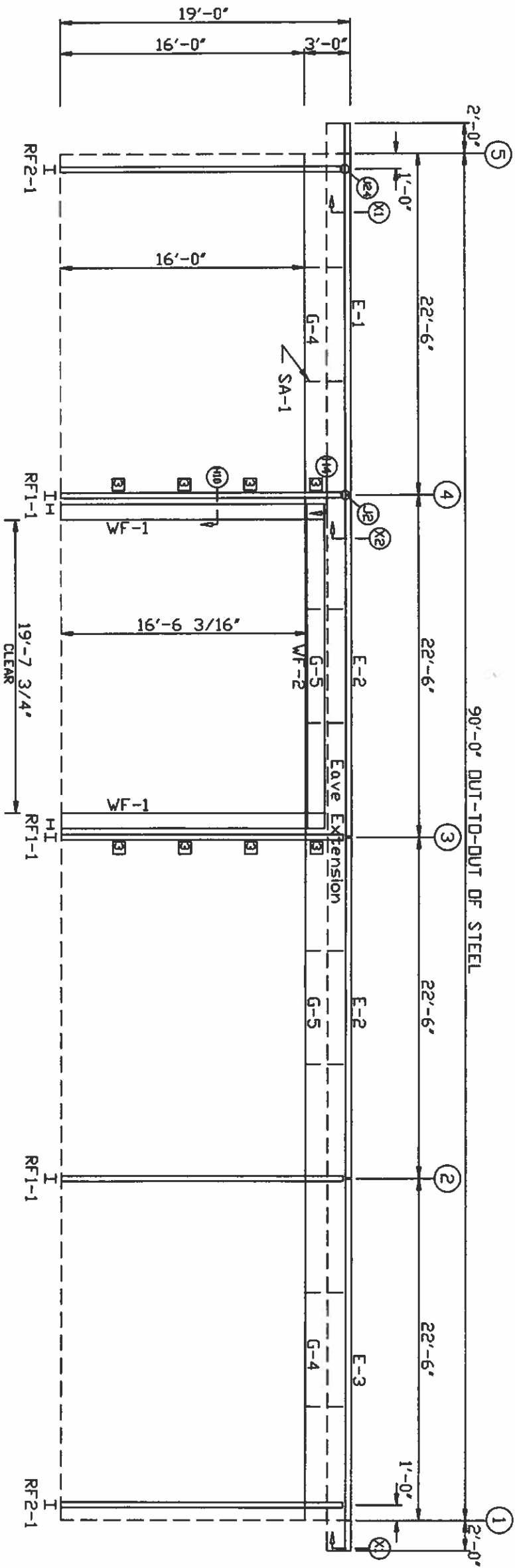
WEB STEEL
01 Washington, LLC
37396 RUBEN LANE
SANDY, OR 97055
OFFICE: (503)-668-7211 FAX: (503)-668-9026

CUSTOMER: Harlie's Angels
PROJECT: Harlie's Hoops Basketball Cover
LOCATION: Longview, Wa 98632
DATE: 7/22/19
DRAWN BY: JSS
JOB NUMBER: R19-021
SHEET NUMBER: E4

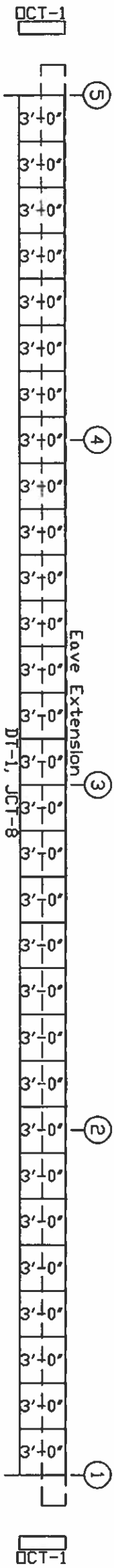
BOLT TABLE		
FRAME LINE B	QUAN	TYPE DIA LENGTH
LOCATION		
WF-1 - WF-2	8	A325 7/8" 2 3/4"
WF-1 - RF1-1	16	A325 5/8" 1 3/4"

MEMBER TABLE		
FRAME LINE B	PART	
MARK		
WF-1	W12X16	
WF-2	W14X30	
E-1	10E3DL14	
E-2	10E3DL14	
E-3	10E3DL14	
G-4	8C16E25	
G-5	8C16E25	

CONNECTION PLATES	
FRAME LINE B	
CD MARK/PART	
3 PC-1	



BACK SIDEWALL FRAMING: FRAME LINE B



BACK SIDEWALL SHEETING & TRIM: FRAME LINE B

PANELS: 26 GA. PR - NEED COLOR



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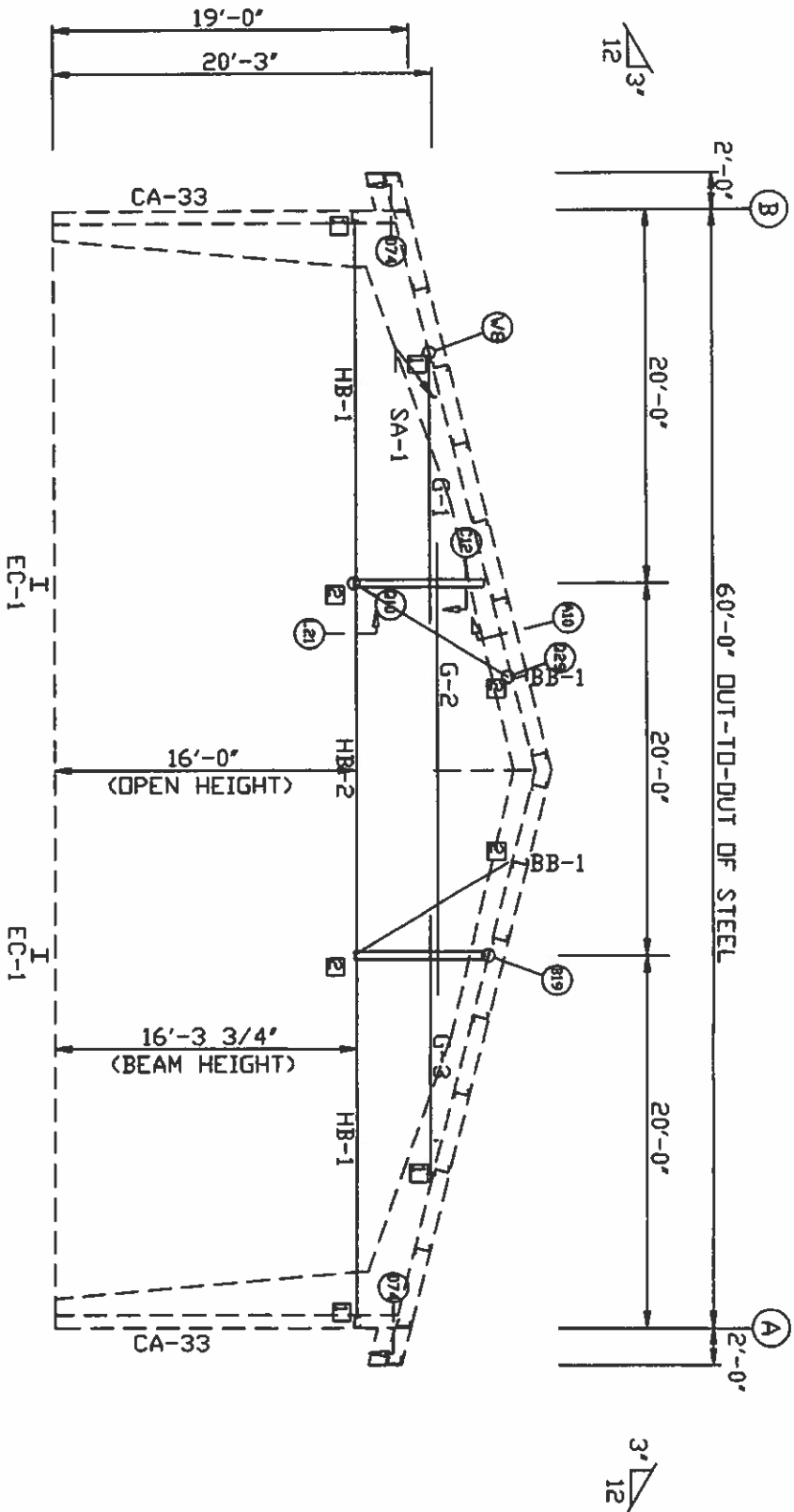
37396 RUBEN LANE
SANDY, OR. 97055
OFFICE: (503)-668-7211 FAX: (503)-668-9026

CUSTOMER: Harlies Angels	
PROJECT: Harlie's Hoops Basketball Cover	
LOCATION: Longview, Wa 98632	
DATE: 7/22/19	DRAWN BY: JSS
JOB NUMBER: R19-021	SHEET NUMBER: E5

BOLT TABLE			
LINE 1	QUAN	TYPE	DIA
LOCATION	2	A325	3/4"
Columns/Raf	1	A307	1/2"
Back Braces	1	A307	1 1/4"

MEMBER TABLE			
LINE 1	MARK	SIZE	
HB-1	8C16E25		
HB-2	8C16E25		
BB-1	8X70D16		
EC-1	W8X10		
G-1	8Z16E25		
G-2	8Z16E25		
G-3	8Z16E25		

CONNECTION PLATES			
LINE 1	MARK	PART	
1	JR-2		
2	JR-1		

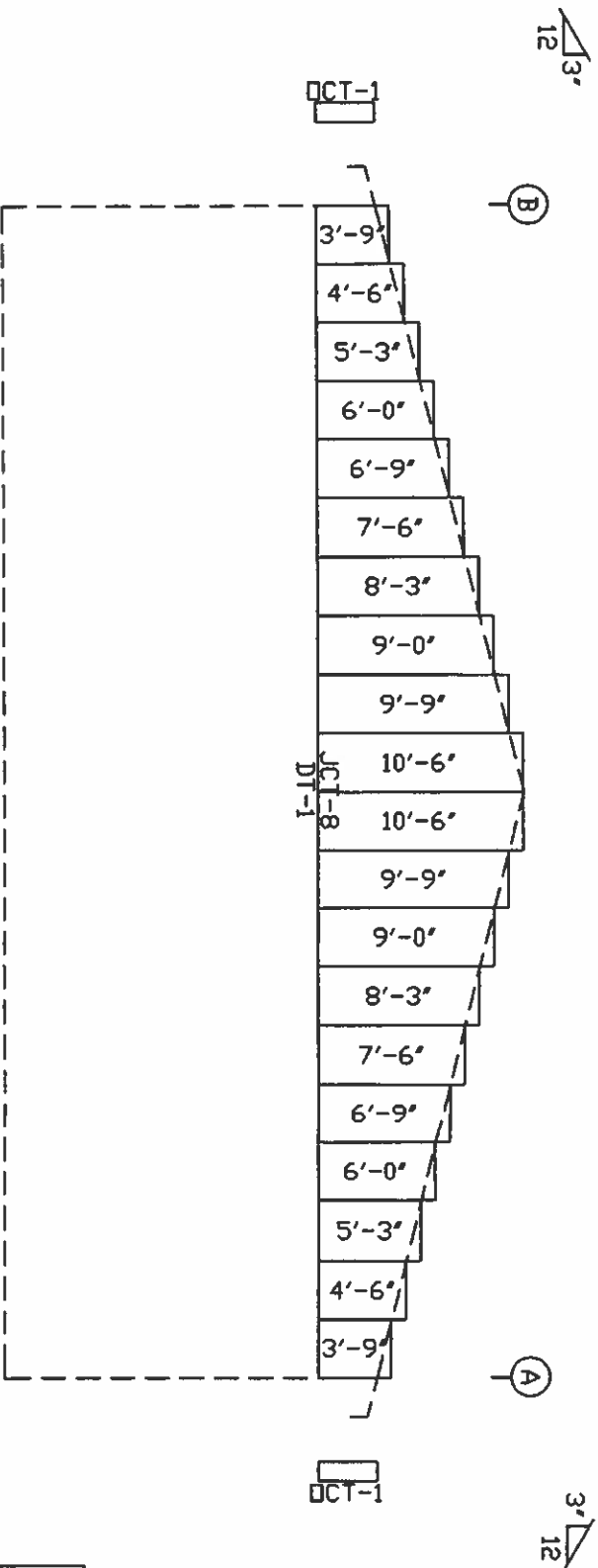


GIRT
LAPS

2'-1 3/4" 2'-1 3/4"

2'-1 3/4" 2'-1 3/4"

LEFT ENDWALL FRAMING: LINE 1



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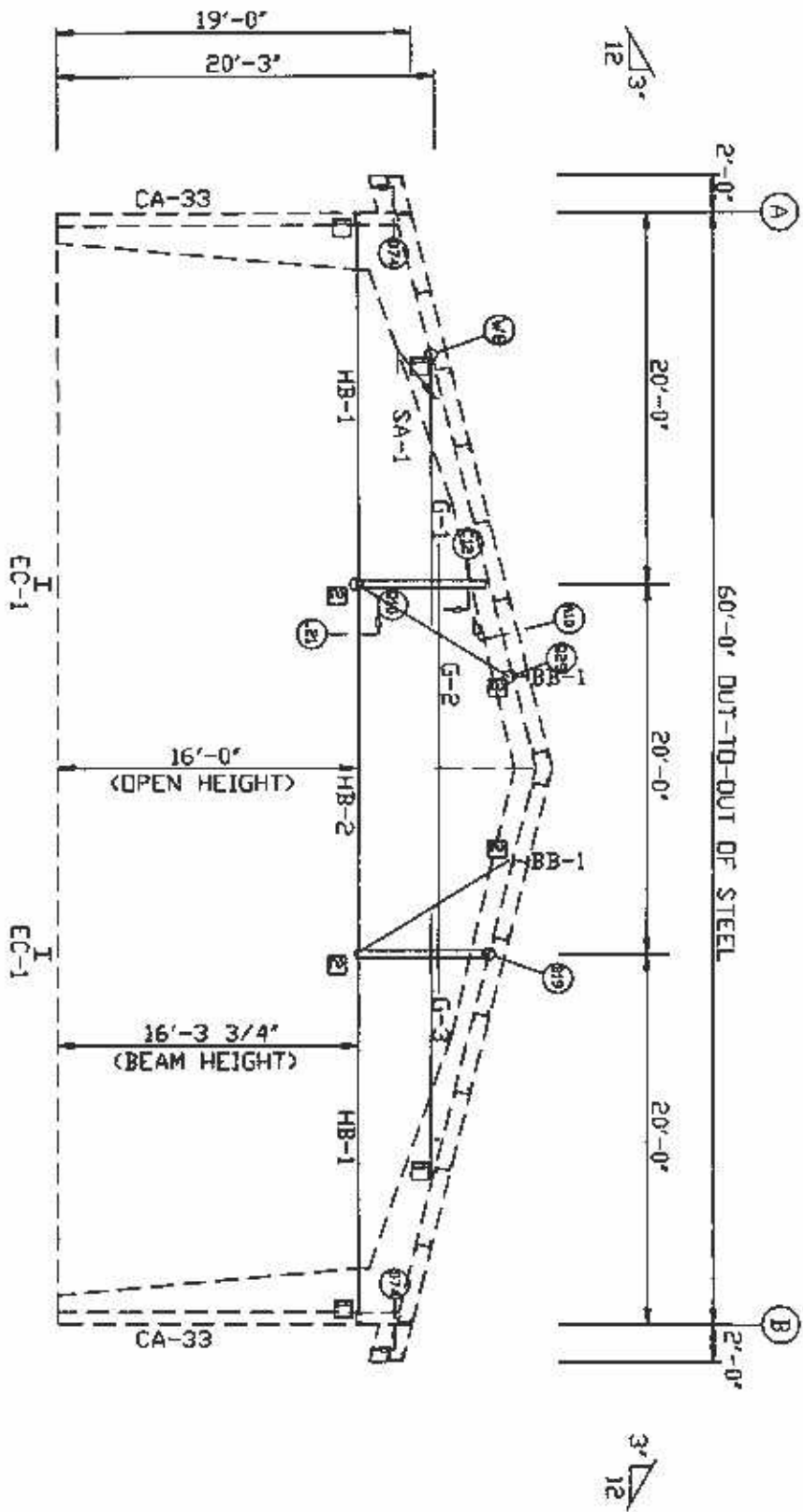
37396 RUBEN LANE
SANDY, OR 97055
OFFICE (503)-668-7211 FAX (503)-668-9026

CUSTOMER: Harlies Angels
PROJECT: Harlies Hoops Basketball Cover
LOCATION: Longview, Wa 98632
DATE: 7/22/19
DRAWN BY: JSS
JOB NUMBER: R19-021
SHEET NUMBER: E6

BUILT TABLE			
LINE	LOCATION	QUAN	TYPE DIA. LENGTH
5	Columns/Raft	2	A325 3/4" 1 1/2"
	Back Braces	1	A307 1/2" 1 1/4"

MEMBER TABLE		
LINE	MARK	SIZE
5	HB-1	8C16E25
	HB-2	8C16E25
	BB-1	8X70D16
	EC-1	W8X10
	G-1	8Z16E25
	G-2	8Z16E25
	G-3	8Z16E25

CONNECTION PLATES	
LINE	MARK/PART
5	1 JR-2
	2 C1

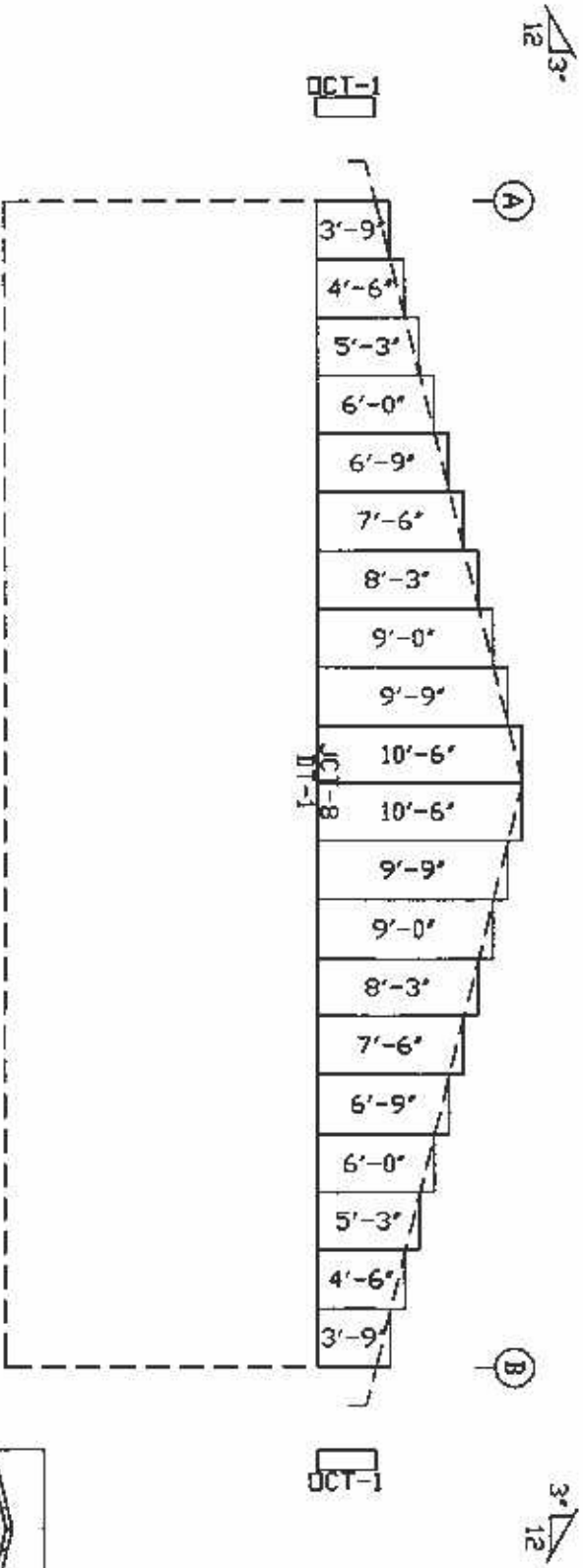


GIRT
LAPS

$$\frac{2'-1 \frac{3}{4}}{2'-1 \frac{3}{4}} = \frac{3/4}{3/4}$$

$$\frac{2'-1 \frac{3}{4}}{2'-1 \frac{3}{4}} = \frac{3/4}{3/4}$$

RIGHT ENDWALL FRAMING: LINE 5



RIGHT ENDWALL SHEETING & TRIM: LINE 5

PANELS: 26 Gd. PBR-PANEL - COLOR



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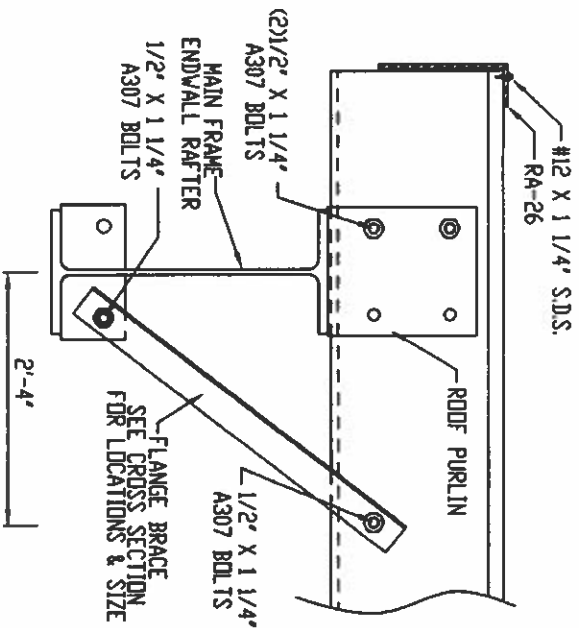
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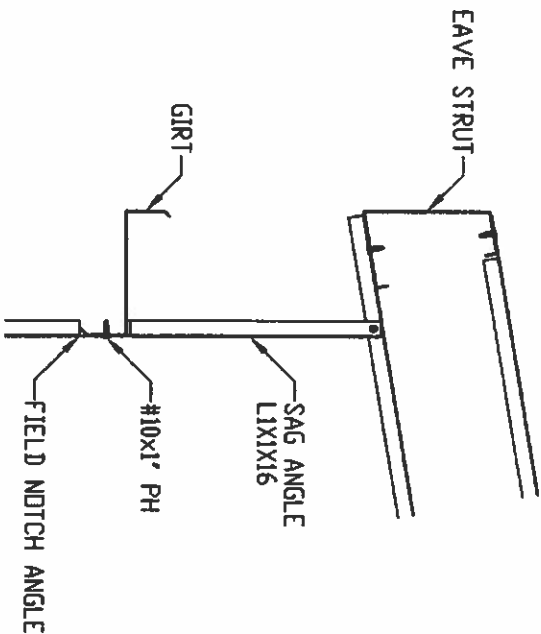


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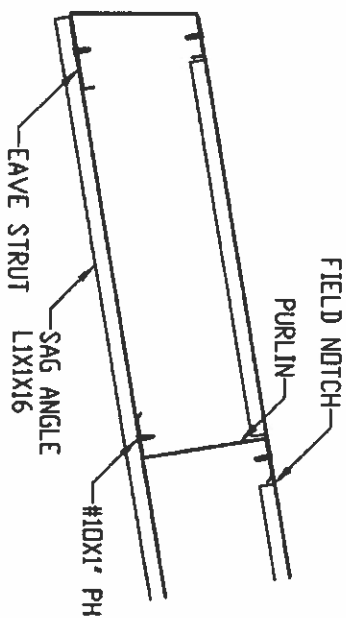
CUSTOMER: Harlies Angels	
PROJECT: Harlies Hoops Basketball Cover	
LOCATION: Longview, WA 98632	
DATE: 7/22/19	DRAWN BY: JSS
JOB NUMBER: R19-021	SHEET NUMBER: E7



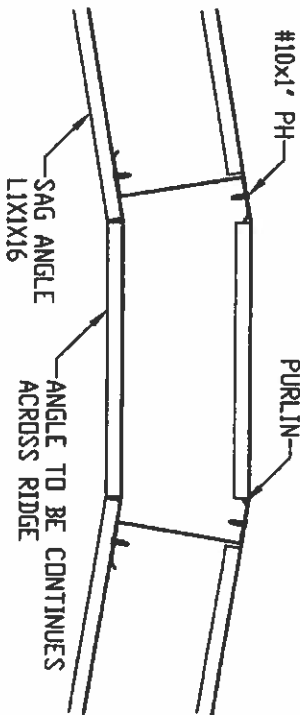
A10 PURLIN TO MF ENDWALL RAFTER



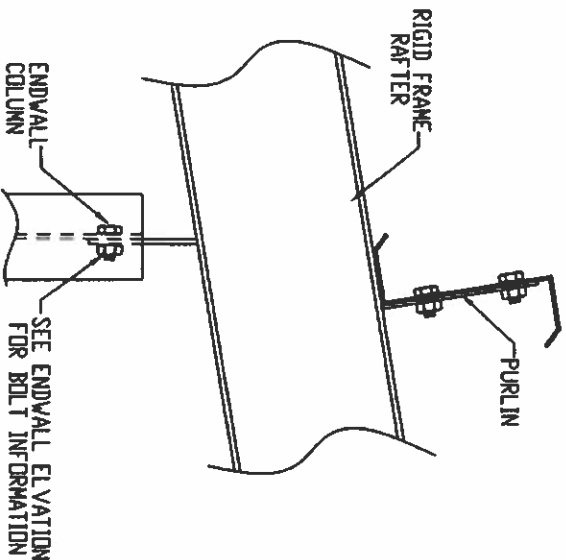
SAG ANGLE @ EAVE (WALL)



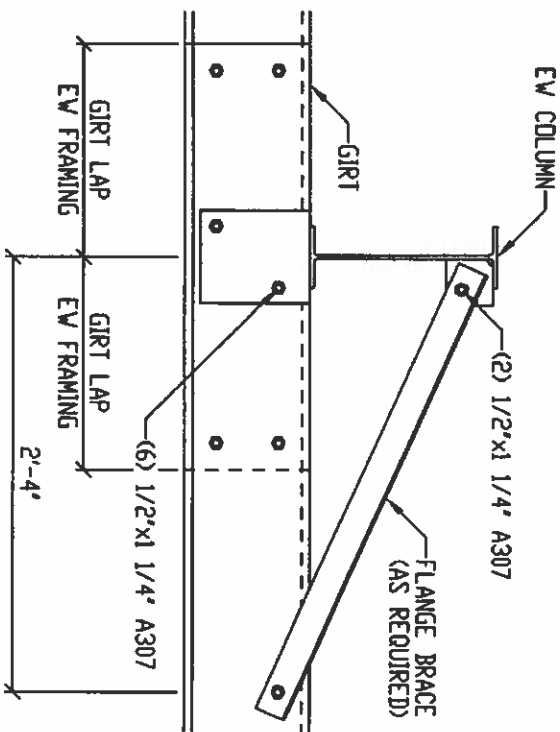
SAG ANGLE @ EAVE (ROOF)



SAG ANGLE @ RIDGE (ROOF)



B19 ENDWALL COLUMN TO EW RF RAFTER



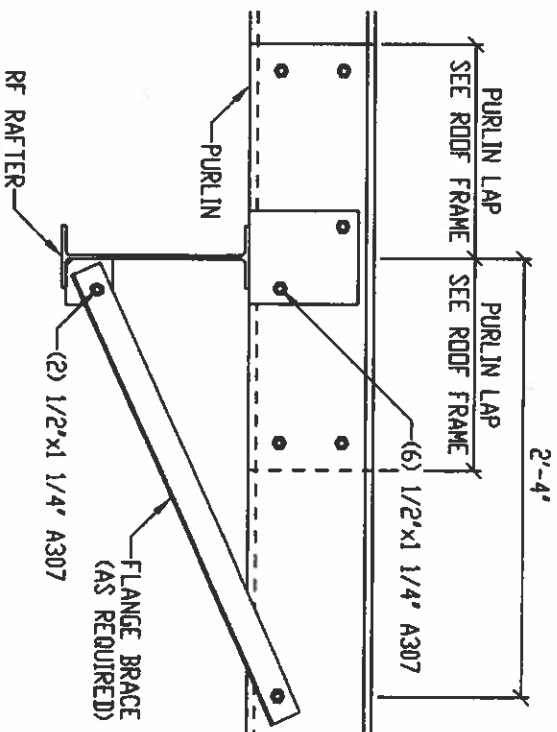
GIRT TO EW COLUMN CONNECTION



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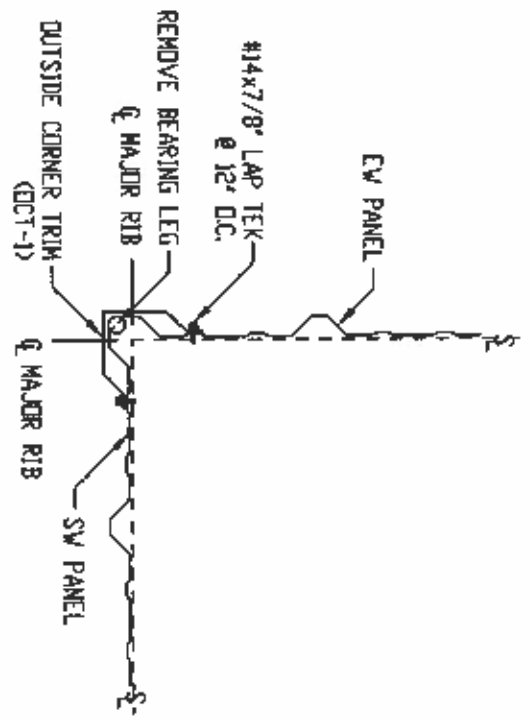


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of Washington, LLC
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OFFICE: (503)-668-7211 FAX: (503)-668-9026

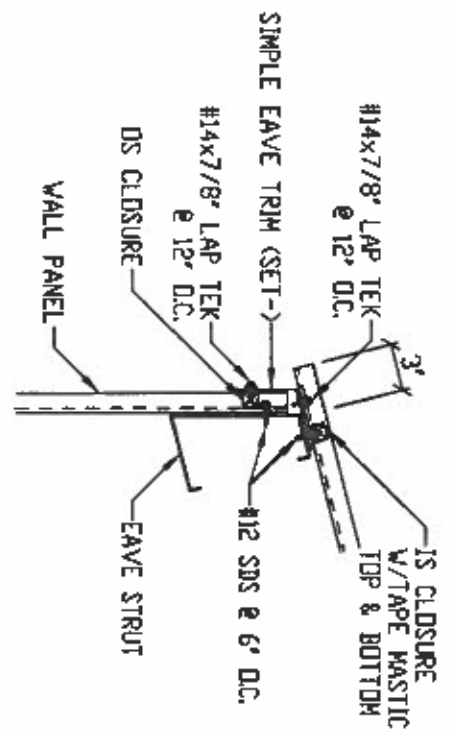


PURLIN TO RF RAFTER CONNECTION

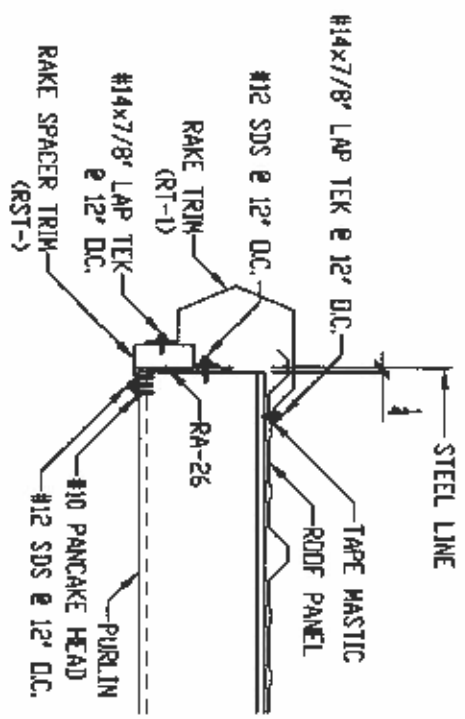
CUSTOMER: Harlie's Angels	
PROJECT: Harlie's Hoops Basketball Cover	
LOCATION: Longview, Wa 98632	
DATE: 7/22/19	
DRAWN BY: JSS	
JOB NUMBER:	SHEET NUMBER: D1
R19-021	



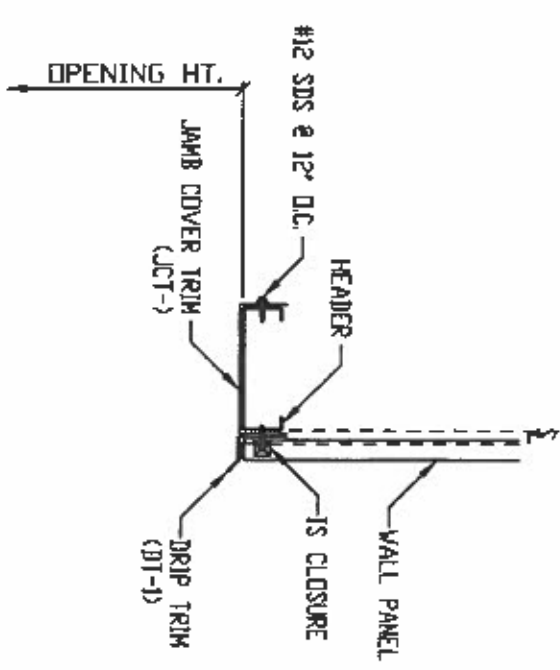
CORNER TRIM



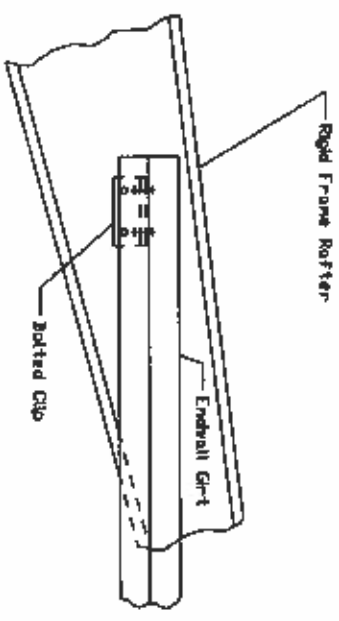
SIMPLE EAVE TRIM



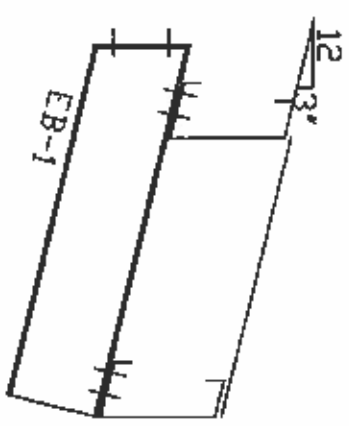
RAKE TRIM (OPEN)



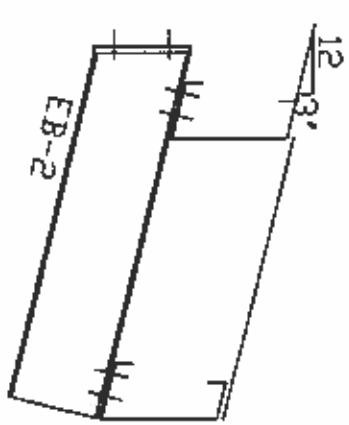
DRIP TRIM



SECTION OF ENDWALL GIRT TO RAFTER



EB-1



EB-2



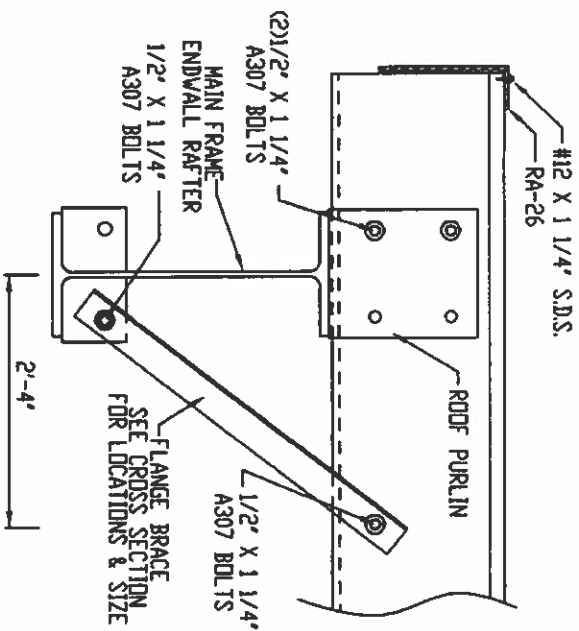
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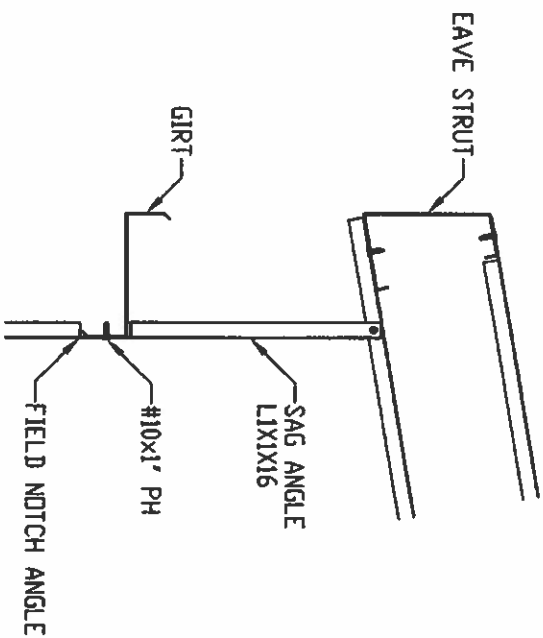


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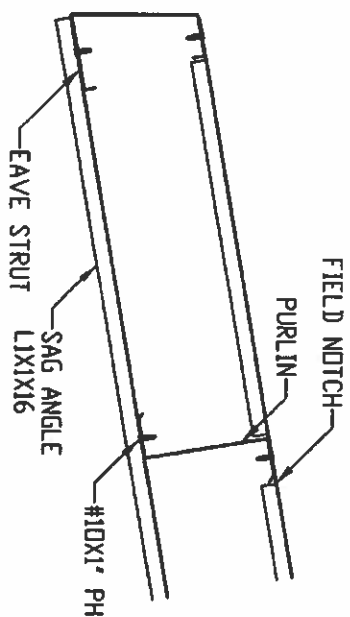
CUSTOMER: Hortle's Angels
PROJECT: Hortle's Hoops Basketball Cover
LOCATION: Longview, WA 98632
DATE: 7/22/19
DRAWN BY: JSS
JOB NUMBER: R19-021
SHEET NUMBER: 03



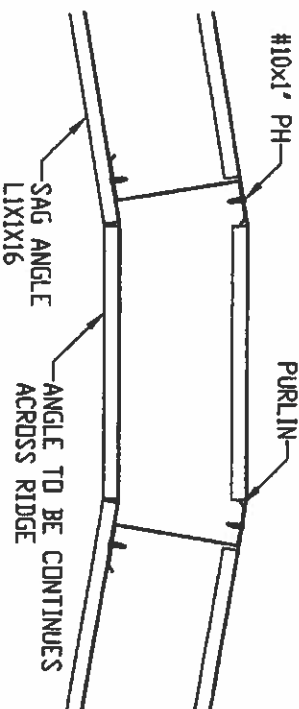
A10 PURLIN TO MF ENDWALL RAFTER



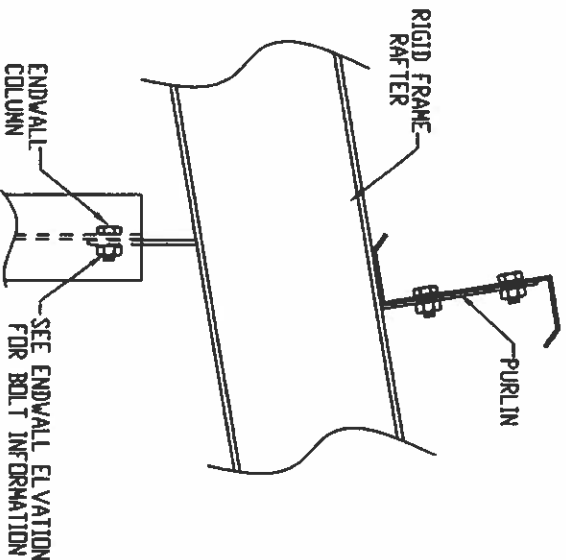
SAG ANGLE @ EAVE (WALL)



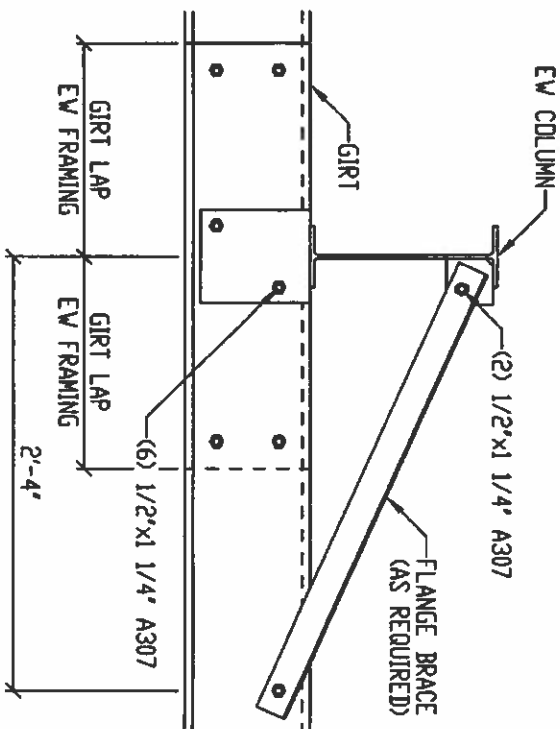
SAG ANGLE @ EAVE (ROOF)



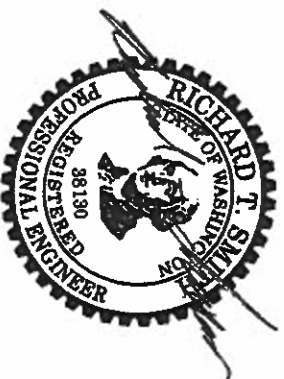
SAG ANGLE @ RIDGE (ROOF)



B19 ENDWALL COLUMN TO EW RF RAFTER



GIRT TO EW COLUMN CONNECTION



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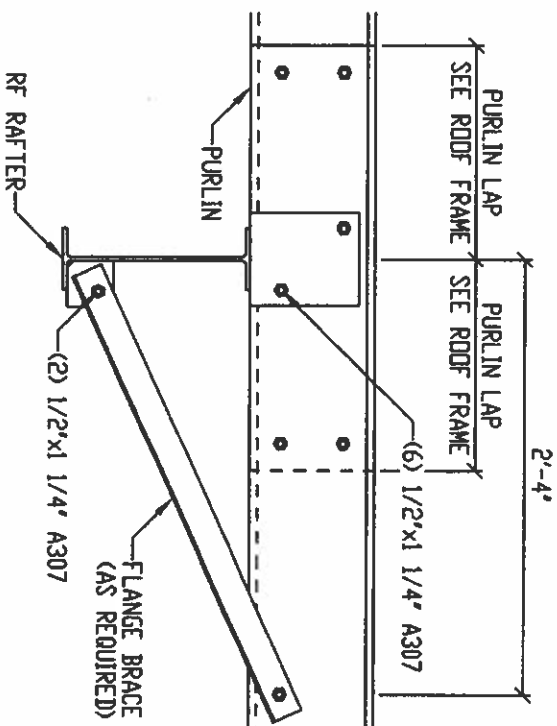
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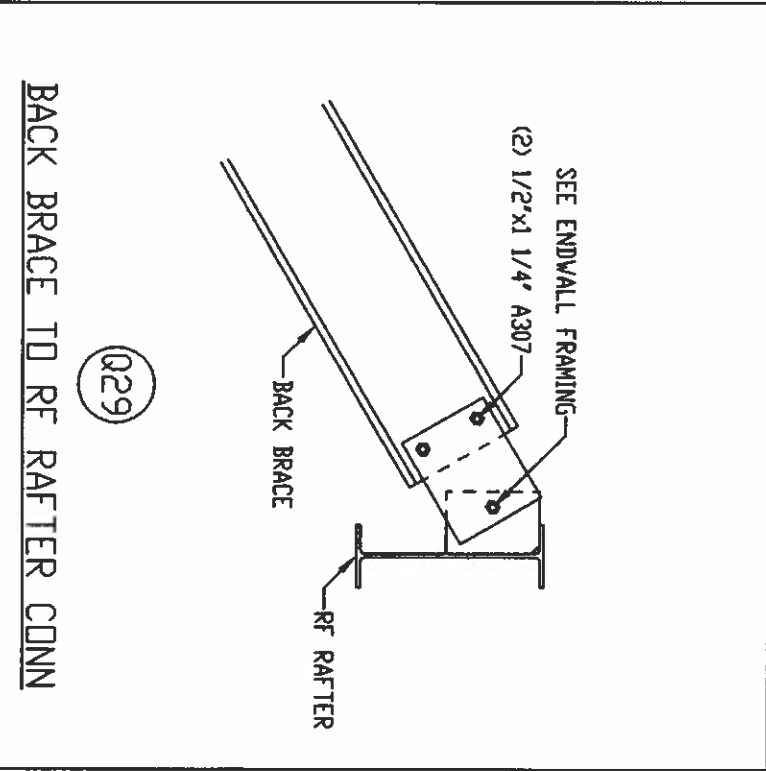
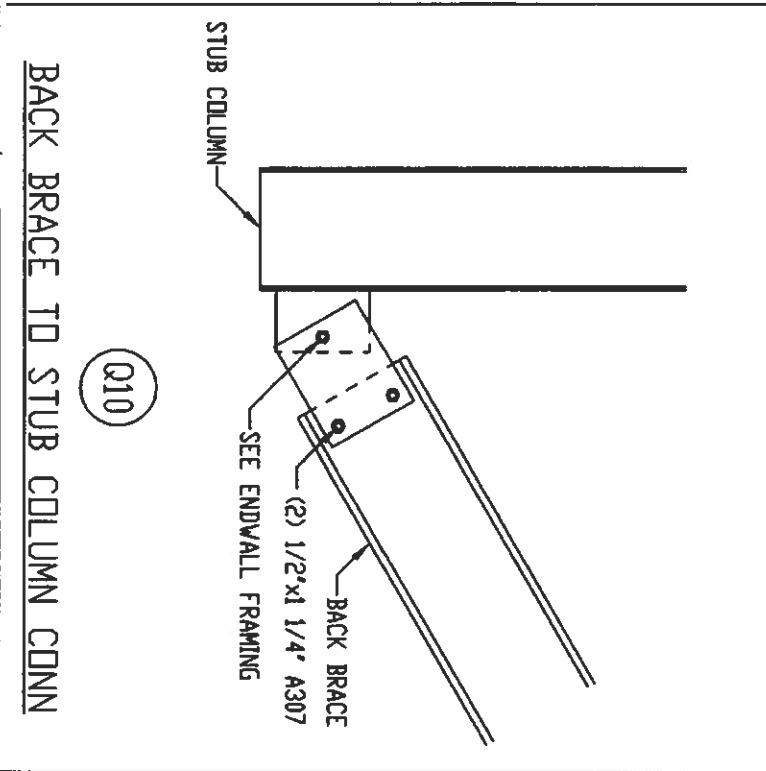
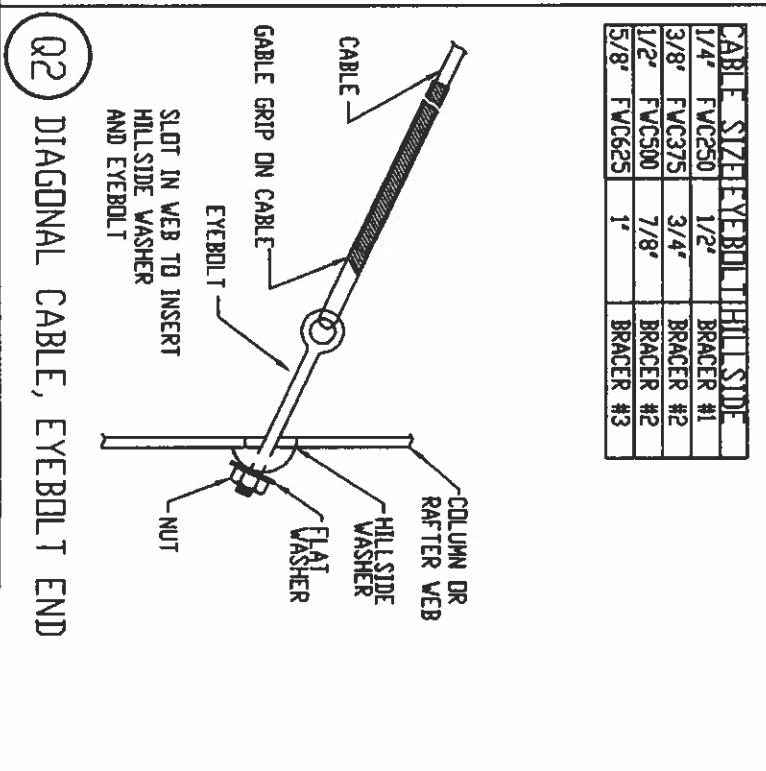
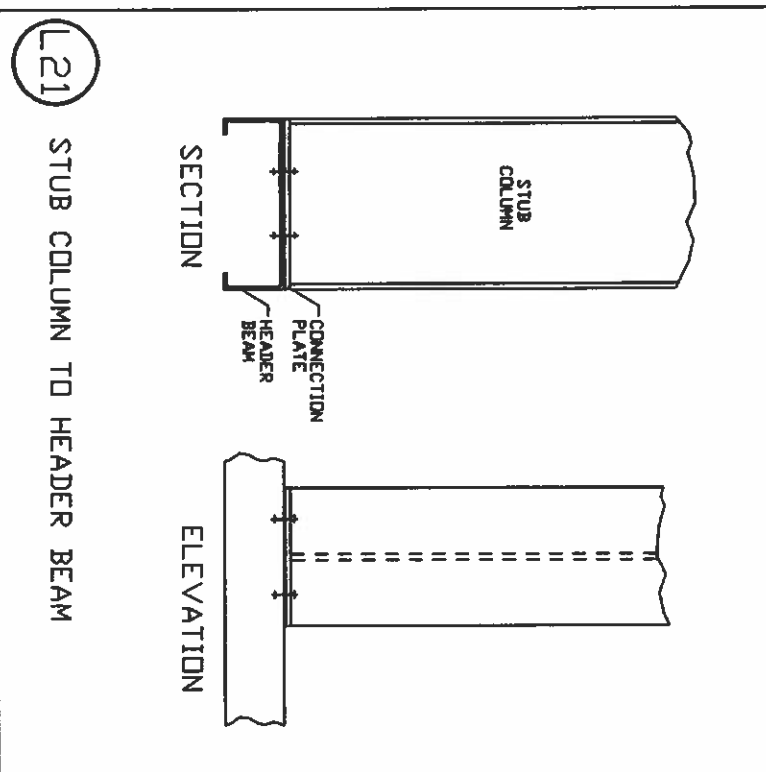
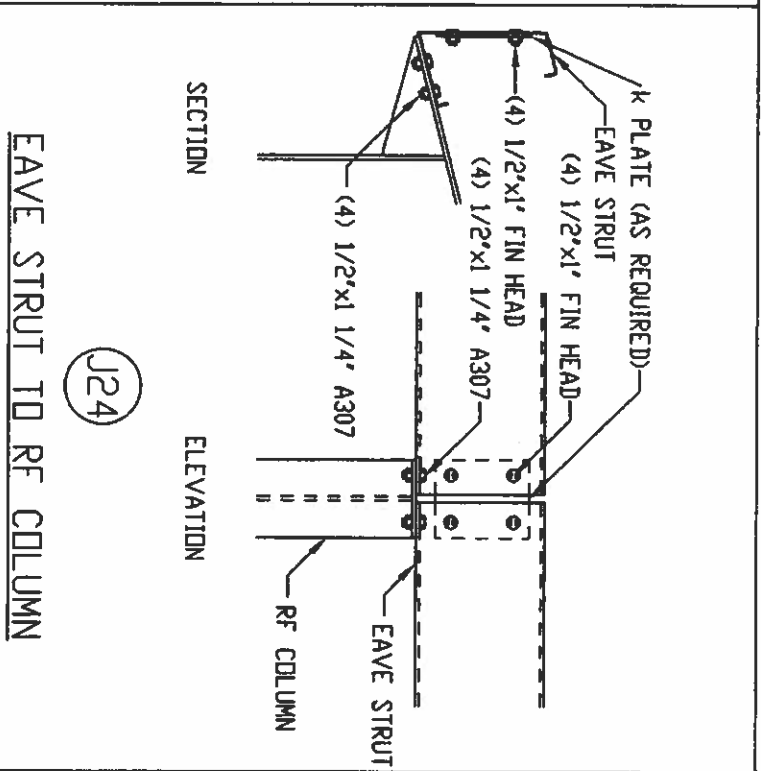
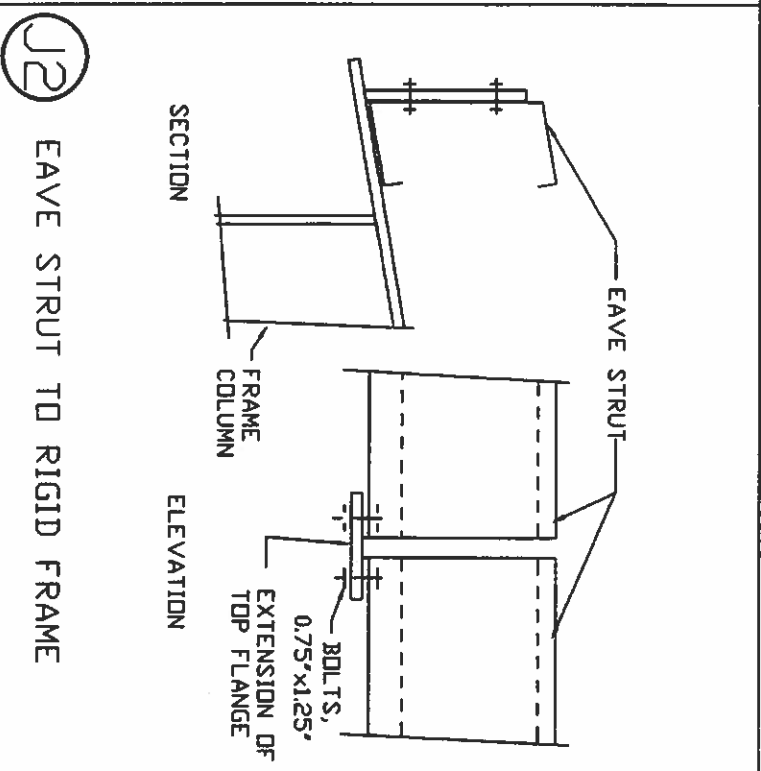
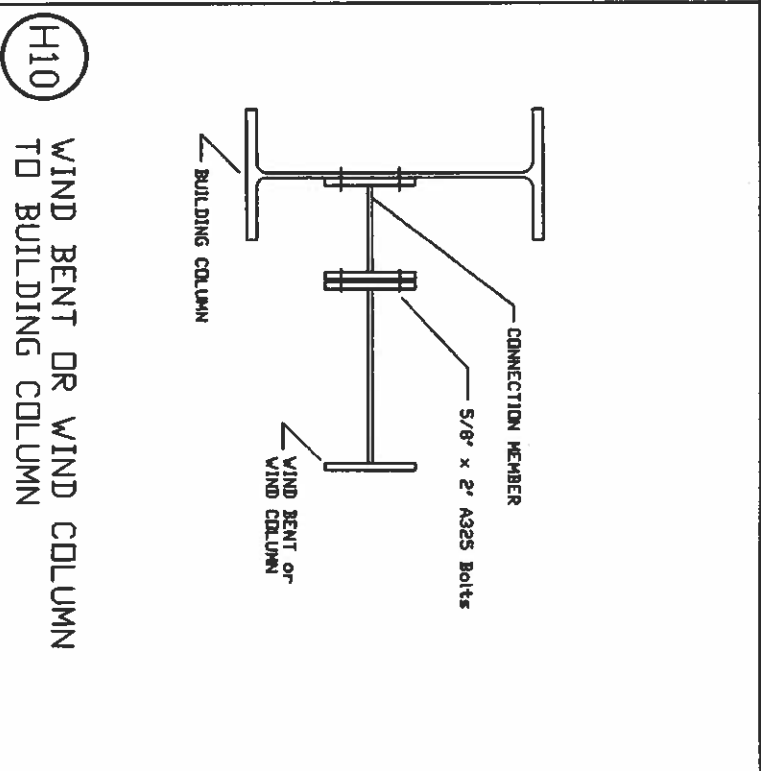
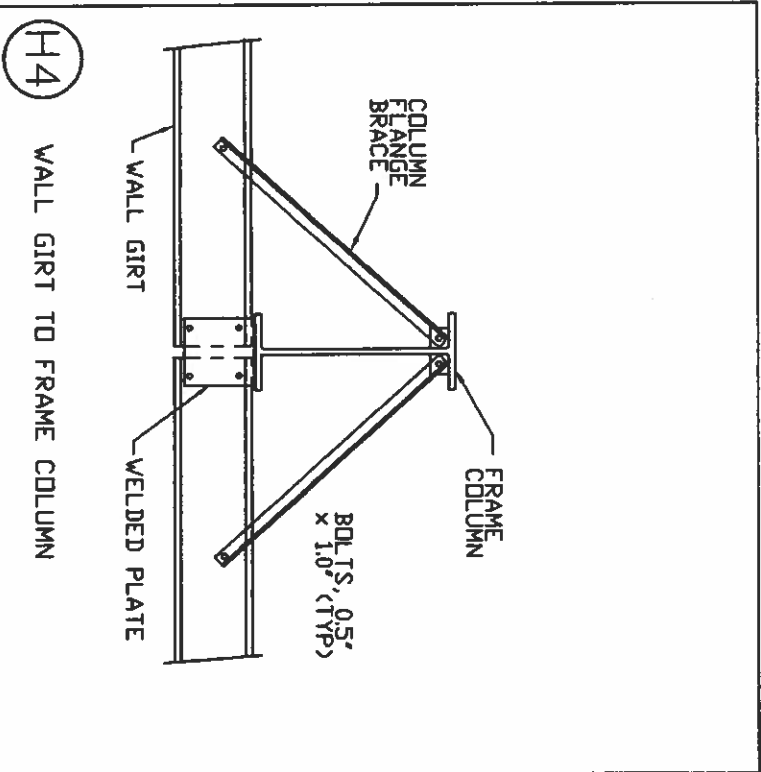
WEB STEEL
L.L.C.
OF Washington, LLC

37396 RUBEN LANE
SANDY, OR, 97055
OFFICE: (503)-668-7211 FAX: (503)-668-9026



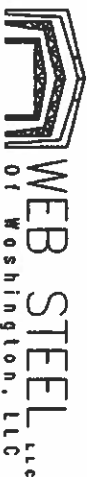
PURLIN TO RF RAFTER CONNECTION

CUSTOMER: Harlies Angels	
PROJECT: Harlies Hoops Basketball Cover	
LOCATION: Longview, Wa 98632	
DATE: 7/22/19	DRAWN BY: JSS
JOB NUMBER:	SHEET NUMBER: D1
R19-021	



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REVIEWED
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SANDY, OR 97055
OFFICE: (503)-668-7211 FAX: (503)-668-9026

CUSTOMER: Harlies Angels
PROJECT: Harlies Hoops Basketball Cover
LOCATION: Longview, Wa 98632
DATE: 7/22/19
JOB NUMBER: DRAWN BY: JSS
SHEET NUMBER: D2



